(Date Received by DNS)



Department of Neighborhood Services Façade Report Application

The following is additional information required to be part of the report per sec. 275-32-13-h:

- A site plan of the building showing adjacent streets and alleys, and relationship of the building to property lines and adjacent buildings.
- A description of the building, including the number of stories, height, plan dimensions, age and type of exterior wall construction, describing (as applicable) cornices, soffits or similar overhangs or features.
- Overall photographs or drawings of the 4 elevations of the building.
- A detailed description of the critical examination in narrative form, including start and completion dates.
- A designation of the building's status by the professional as stated above.
- Drawings or photographs describing the locations and extent of all significant distress or deteriorated conditions observed in the facades.
- A description of recommended repair work and precautionary measures that will be taken to safeguard the public, if any, and the recommended completion date of such work.
- Where appropriate, a comparison of conditions of facades on the building with conditions observed during previous examinations.
- A recommendation for future examination, if earlier than 5 years from date of the report.



I. General Information

Contacts

Building Owner: Milwaukee County Department of Administrative Services

Address: 901 North Ninth Street

Courthouse Room 308

Milwaukee, Wisconsin 53233

Contact: Philip Schmidt Telephone: 414-278-4936

Building Agent: N/A

Professional: Klein and Hoffman, Inc.

Address: 150 South Wacker Drive, Suite 1900

Chicago, Illinois 60606

Contact: David Weirick
Telephone: 312-251-1900

Building Description

Building Age: 86 Years Façade Age: 86 Years Stories: 9 stories

Height: 255 feet (grade level to upper penthouse parapet copings)

Plan Dimensions: 210 feet (east/west) x 435 feet (north/south)

Façade Type: Limestone with aluminum grillages, bronze entrances, and metal

windows.

Cornices: Stone cornices located at the 6th floor on the main building and the 8th

mezzanine level of the upper penthouses.

Soffits: Stone soffits at the 6th floor level, top of colonnades, and at arched

windows above entrances at the first floor.

Site Plan

Refer to site plan and inspection plan in Appendix A.

Building Elevations

Refer to elevation drawings in Appendix B.



Review of Previous Reports

The following reports were reviewed a part of the Critical Examination:

1. City of Milwaukee Façade Ordinance / Critical Examination Report

Milwaukee County Courthouse 901 North 9th Street Milwaukee, Wisconsin 53233

Date: December 2, 2008

By: Graef Anhalt Schloemer & Associates, Inc.

2. Façade Evaluation

Milwaukee County Courthouse 901 North 9th Street Milwaukee, Wisconsin 53233

Date: June 8, 2010

By: Inspec

3. Summarization of Work Performed

Milwaukee County Courthouse 901 North 9th Street Milwaukee, Wisconsin 53233

Date: December 19, 2010

By: Inspec



II. Description of Critical Examination

Contractor for the Critical Examination

Masonry Restoration Incorporated

9522 W. Schlinger Avenue Milwaukee, Wisconsin 53214

Phone Number: (414) 259-8111 Contact: Tony Lipek

Start and Completion Date of Examination:

Start Date: August 15, 2016 Completion Date: September 20, 2016

Number of Drops: 46 scaffold drops and close-up inspection from a 185' high-reach lift.

Refer to Site/Drop Location Plan in Appendix A

Narrative Description

Close up inspections were made from a 185' high reach lift at 100% of the west elevation and the west ends of the north and south elevations. Swing-stage scaffolds were used to complete the close-up inspections at 100% of the remainder of the north and south elevations, the east elevation, and the north and south façades above the sixth-floor roof level.

Areas on the façades exhibiting visual signs of distress were sounded with metal hammers in an effort to determine the extent of distress.

Inspection drop sheets indicating areas of distress on the façades can be found in Appendix C. Representative photographs of façade conditions encountered during inspections can be found in Appendix D.

Select locations where sealant failures were observed to be widespread are depicted in Appendix E.

The extents of old and replacement built-in sheet metal gutters, as well as areas where ponding water indicate clogged roof drains are depicted in Appendix F.

Building Status Designation:

North Elevation: Safe* with an ordinary repair and maintenance program. Safe* with an ordinary repair and maintenance program. South Elevation: Safe* with an ordinary repair and maintenance program. West Elevation: Safe* with an ordinary repair and maintenance program.

^{*} As defined by the Façade Ordinance, Rules and Regulations, Number i-2.



III. Findings and Conclusions from the Critical Examination

- Refer also to the Critical Examination Inspection Drop Sheets in Appendix C.
- Photographs referenced below can be found in Appendix D.

A. Distressed Limestone Masonry Conditions

The majority of the 300,000+ sq. ft. limestone façades are in fair to good condition, defects in the stonework were found at random locations, they included:

- 1. Spalled sections of stonework had been previously removed at quite a few locations (see Photo No. 1). New spalls were removed at numerous locations during our inspections.
 - a. Approximately eight (8) small spalls were observed at the high, eighth floor, cornice. The spalls appeared random, and did not seem to suggest systemic deterioration of concealed conditions (see Photo No. 2).
 - b. Approximately sixteen (16) small to medium sized spalls were observed at the sixth floor cornice. These spalls generally correspond with areas where the sheet metal gutter does not appear to have been recently repaired or replaced. Cracked solder seams and standing water were common, suggesting that water infiltration through the gutter, into the masonry and subsequent freeze/thaw damage is the cause of the spalls (see Photo No. 3).
 - c. Approximately twenty four (24) small to medium sized spalls were observed at the ashlar base of the building floors one through three. These spalls were concentrated below the cornice at the base of the colonnade where open wash joints were common, suggesting that water infiltration through these joints, into the masonry below, is causing corrosion of embedded metal framing and anchorages (see Photo No. 4).
 - d. Several spalls were observed at the southeast corner below the eighth floor cornice (refer to drop sheet from drop nos. 31-35). Corroded metal cramp anchors were observed after removal of the loose spalls. Building corners, in general, are exposed to the worst weathering, and are also the most susceptible to open joints cause by building movement. These factors, combined with open wash joints at the eighth floor cornice have likely contributed to water infiltration into the masonry and corrosion of embedded metal anchorages as well as freeze/thaw damage (see Photo No. 5).
 - e. Spalls were frequently observed at the stone surrounding entrances. Such spalls appear to be due to exposure to high traffic, de-icing salts, and general weathering (see Photo No. 6).



- 2. Separations in mortar joints, indicating ongoing movement of the stone units, is generally caused by water trapped within the walls during freeze/thaw cycles or by ongoing corrosion of embedded steel components within the walls. This condition occurs at random locations throughout the facades (see Photo Nos. 7 and 8).
 - a. This condition was especially pronounced at the parapet at the northeast corner, where coping and parapet stones have shifted due to expansion of saturated mortar joints during freeze/thaw cycles (see Photo No. 9).
 - b. This condition was observed at most corners at the eighth floor cornice. Deterioration of mortar joints and subsequent movement of stone units is normal for such locations, where exposure to weathering is most severe (see Photo No. 10).
 - c. Some horizontal (non-skyward facing) mortar joints, mainly at cornices, were observed to be sealed. Sealing of horizontal mortar joints, especially at projecting masonry, is not typically recommended because it traps moisture in the masonry (see Photo No. 11).
- 3. Continuous cracking, generally vertically, in stone units was found at several locations and indicates distress within the wall at those areas (see Photo Nos. 12 and 13).
 - a. At the northeast and southeast corners of the seventh through eighth floors, numerous cracked stones were observed. Cracking also extended into the brickwork below the stone. The cracking may be due to corrosion of corner steel columns, and the subsequent transfer of expansive forces into the masonry (see Photo Nos. 14 and 15).
 - b. Cracked stones were commonly observed at the bottom of the returns at the ground level arcade. The cracking may be due to corrosion of corner steel columns. Steel columns are especially susceptible to corrosion at their bases where they meet the ground, where masonry is often saturated with water for extended periods of time (see Photo No. 16).
- 4. Hundreds of cracked stones throughout the facades had been previously patched or repaired. Poorly matching patch material was frequently observed and no supplemental anchors were found to have been installed in the patch repairs which were removed during inspections. Defective patches were removed at numerous locations (see Photo Nos. 17 and 18).
- 5. Crack repairs were generally in fair to good condition. Isolated locations where new cracks had developed at previously repaired cracks were observed (see Photo Nos. 19 and 20).
 - a. Previous pin and epoxy repairs of cracked stones were observed at the north and south corners of the central portion of the east elevation (refer to drop sheets from drop nos. 10 and 13a), and at various locations along the sixth floor cornice. The repairs appeared to be in good condition (see Photo Nos. 21 and 22).



- 6. Cracked stones were observed at several locations at the base of columns at the fourth floor colonnade. The cracking appears to be due to differential movement due to the great weight of the columns (see Photo No. 23).
- 7. Previous limestone cleaning efforts with harsh acidic cleaning materials have resulted in damage to the outer finished face of the stonework at numerous locations (see Photo No. 24). Rust staining of large portions of the stonework on the east elevation is the result of cleaning with harsh chemicals which reacted with the iron deposits in the stone. The surface of such stones was generally stable, however a small amount of flaking and delaminating surface was removed where observed (see Photo No. 25).
- 8. Some areas of surface erosion were observed at the south side of the east elevation. This erosion appears to be due to localized severe weathering. Such surfaces were generally stable, but loose material was removed where observed (see Photo No. 26).
- 9. The mortar joints in the stonework on the façades are generally in fair condition (see Photo No. 27).
 - a. Isolated deteriorated mortar joints were observed throughout the facades. Failed mortar joints were frequently observed at stacked joints at the corners of pilasters, corners of the ground level arcade, and projecting masonry (see Photo Nos. 28, 29, and 30).
 - b. During what appears to be the most recent tuck pointing effort on the west elevation, no attempt was made to match the existing mortar color. The joints appear to be repointed with an almost white mortar, which stands out noticeably (see Photo No. 31). This may be the same material that was used to patch cracks in the stone units.
- 10. At the south side of the east elevation, deterioration of the stone surface at the underside of the sixth floor cornice was observed. The deterioration appeared to be superficial, and is likely related to past water infiltration through the sheet metal gutter directly above. Loose surface material was removed where observed (see Photo No. 32).
- 11. Significant deterioration of stonework was observed at building entrances. Cracks, spalls, open mortar joints, and surface erosion were observed at these locations. Separation of mortar joints and stone displacement at the base of the stone piers flanking the revolving door entrances suggests corrosion of underlying steel framing (see Photo No. 33).
- 12. Severely deteriorated and/or failed sealant on wash (upward facing) joints were found throughout the façades (see Photo Nos. 34 and 35). Water freely entering into the building's façades will promote advanced deterioration of the façade components. Water trapped in the walls during freeze/thaw cycles will generate expansive forces causing spalling and cracking of masonry units (see Photo Nos. 36 and 37). Water entering the walls will also promote corrosion of embedded steel components such as lateral ties, anchors, and support angles and possibly the structural steel framework. The corrosion of embedded steel components within the walls generate expansive forces to the exterior masonry resulting in spalling, cracking, and displacement (see Photo Nos. 38 and 39).



- 13. Small epoxy repairs were observed at many locations throughout the facades. The repairs were generally in good condition (see Photo No. 40).
- 14. Stone Dutchman repairs were observed at many locations throughout the facades. The repairs were generally in good condition (see Photo No. 41).

B. Additional possible points of water entering into the façades include the following:

- 1. Open seams in the metal gutter atop the cornice at the 6th floor were found randomly throughout the façades (see Photo Nos. 42 and 43). The gutter had been previously replaced at the west and south end of the east elevations. The open seams appear to be the result of normal expansion and contraction of the gutter system during thermal cyclical changes, and were more common at areas where the gutter had not been recently repaired or replaced. Ponding water in the gutters is likely to infiltrate into the facades through the open seams.
- 2. Back-pitched weep holes installed directly over the back side of the gutter occur at the south end of the west elevation (see Photo No. 44). Ponding water in the gutter below indicates a clogged drain in the area (see Photo No. 45). Water overflowing the gutter during heavy rains will flow into the walls through the back-pitched weep holes prompting accelerated deterioration of the façades below.
- 3. Open and deteriorated sealant joints at the metal collector pans between column bases at the 4th floor colonnades (see Photo No. 46).
- 4. Window perimeter sealant joints were generally in poor condition on the north east and south elevations, with open/failed joints commonly observed (see Photo No. 47). Perimeter sealants on the west elevation were in fair condition, apparently having been replaced in the not too distant past.
- 5. Sealant joints at roofing membrane perimeter counter flashings are in poor condition. Failed and open joints were commonly observed and allow water infiltration into the masonry below.

C. Brick Masonry

The brickwork on the façades occurs behind parapet walls at the base of the set back upper floors, the setback rooftop penthouses, and the interior of the light courts. The majority of these areas are not considered part of the exterior façades and were not closely reviewed as part of the Critical Examination of the exterior façades.

1. Large cracks were observed at the northeast and southeast corners of the sixth floor, below the limestone (refer to drop sheets from drop nos. 21 and 36). These cracks may be due to the normal stresses caused by movement of walls, and may be exacerbated by corrosion of embedded steel columns (see Photo No. 48).

D. Parapets

1. Limestone masonry of the various parapets was generally in good condition. Minor cracks and spalls were observed at isolated locations at all parapets (see Photo Nos. 49 and 50).



- As previously discussed, the parapet at the northeast corner of the sixth floor is in poor condition. Open wash joints have led to water infiltration of the masonry and subsequent movement due to freeze/thaw cycling. Water infiltration and movement have also resulted in numerous small cracks and spalls in this area (Photo Nos. 51).
- 3. At the southwest parapet of the sixth floor and at the south and west parapets of the high roof, significant weathering of stone surfaces was observed, primarily at the tops of walls. This observation corresponds with weathering observed at the outside face of the parapets in these locations (see Photo No. 52).
- 4. At the seventh floor parapets (north and south) small spalls were frequently observed where sheet metal cap flashings were fastened (see Photo No. 53).
- 5. Mortar joints at parapets were generally in good condition. Isolated deteriorated mortar joints were observed.
- 6. Sealant joints at the parapets and copings were generally in poor condition. Failed/open sealant joints were frequently observed (see Photo No. 54).

E. Windows and Grillages

- 1. Above the first floor the windows are replacement windows with metal frames and are in fair to good condition (see Photo No. 55).
 - a. Glazing seals are in fair condition.
- 2. Metal grillage is installed over the 5th floor windows, in an apparent effort to deter roosting birds, and is in fair to good condition (see Photo No. 56).
- 3. At the first floor, large arched windows with faux entry portals, are set back from the face of the façades and are in fair condition. The windows in these locations are replacements, however the original bronze windows remain at the three main entrance portals at the east elevation. The bronze windows are in good condition, although have advanced patina. (see Photo Nos. 57 and 58).



IV. Recommended Repair Work

The following conditions, found to be potentially "unsafe and imminently hazardous" in the near future were promptly removed or stabilized by the masonry contractor at the direction of K&H:

- At the southwest corner of the seventh floor cornice, two (2) stainless steel pins
 were installed at a cracked stone. The pins consist of stainless steel threaded
 rods set in epoxy in the back-up masonry, with a mechanical attachment
 provided by stainless steel nuts and washers.
- At the northeast and southeast corners of the seventh through eighth floors, stainless steel straps were installed to stabilize cracked stones. The stainless steel straps were secured with stainless steel threaded rods set in epoxy in the back-up masonry with a mechanical attachment provided by stainless steel nuts and washers. Cracks in the limestone and brick below were sealed (see Photo No. 59).
- 3. At the northeast sixth floor parapet, eight (8) cracked stones were pinned in place with using the same method as described in item no. 1 above. Cracks in the limestone and all open wash joints within reach were sealed (see Photo No. 60).
- 4. At the pilaster at the east end of the north elevation, one large spall was removed and pins were installed at the edge of the spall to temporarily stabilize the stone. Pins were also installed at a similar, previously removed spall, further down on the pilaster (see Photo No. 61).

Recommendations for repairs of distressed conditions and completion dates for repair are discussed below. K&H recommends that the owner consider performing the repairs which are recommended to be completed within two (2) years at the same time as the repairs which are recommended to be completed within one (1) year to save mobilization costs.

- 1. Where the above described stabilization repairs were completed, permanent repairs should be completed within six (6) months to one (1) year. Such repairs would likely include replacement of cracked stones, partial rebuilding of the northeast parapet, and Dutchman repairs or stone replacement at large spalls.
- 2. The primary cause of the distressed conditions observed on the façades is water entering into the building's façades. Based on previous reports, it appears that water entering into the façades was also the primary cause of the spalled stone that fell from the west elevation in May of 2010. K&H recommends that the sealant on 100% of the wash (upward facing) joints be replaced within one (1) year.

The building may want to include the installation of weather cap joint protection along with the wash joint sealant replacement. Weather cap joint protection consists of lead 'T's that embed into the sealant joints and prolong the life of the sealant joints for extended periods. We highly recommend the weather cap joint protection, particularly for buildings with difficult, and costly, access to the façades.

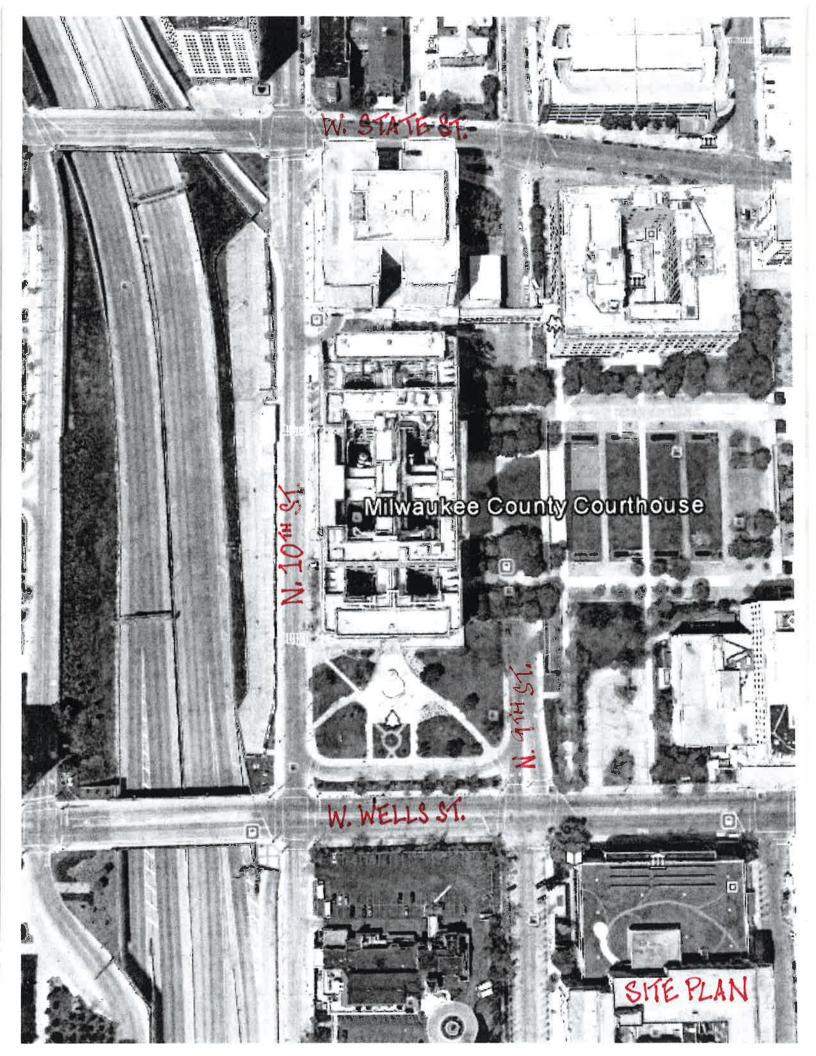


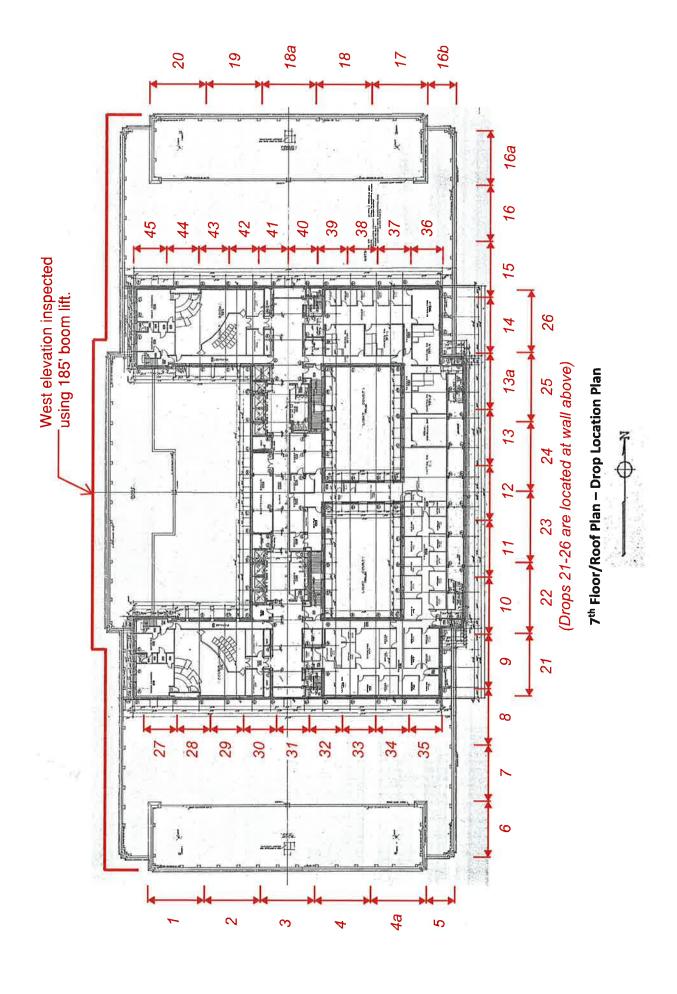
On the upper portion of the façade at the center of the east elevation, two of the wash joints on the parapet copings had weather cap protection installed (see Photo No. 62). The installation of the weather cap joint protection may well have been incorporated in the original construction of the courthouse. We have observed weather cap installation on numerous buildings of this vintage, particularly governmental and institutional buildings.

- 3. Sealant joints at the window perimeters on the north, east and west elevations, as well as the roof counter flashings should be replaced within one (1) year.
- 4. Open seams in the inlaid copper gutter atop the cornice should also be repaired within one (1) year. Detailing of the seam repairs will need to address the expansion and contraction of the gutter system.
- 5. The back pitched weep holes directly above the cornice gutter on the west elevation should be properly reinstalled or abandoned and sealed within one (1) year.
- 6. Spalled, displaced (shifted). And cracked stone units should be repaired or replaced within the next two (2) years. These repairs would involve removal and repair or replacement, or merely removal and reinstallation of intact displaced stone units. These repairs will likely include repairs/replacements of embedded steel components and rebuilding of back-up masonry.
- 7. Open and/or severely deteriorated mortar joints should be repaired and repointed within the next two (2) years.
- 8. Cracked, shifted, and severely deteriorated brick masonry should be rebuilt within the next two (2) years.

APPENDIX A

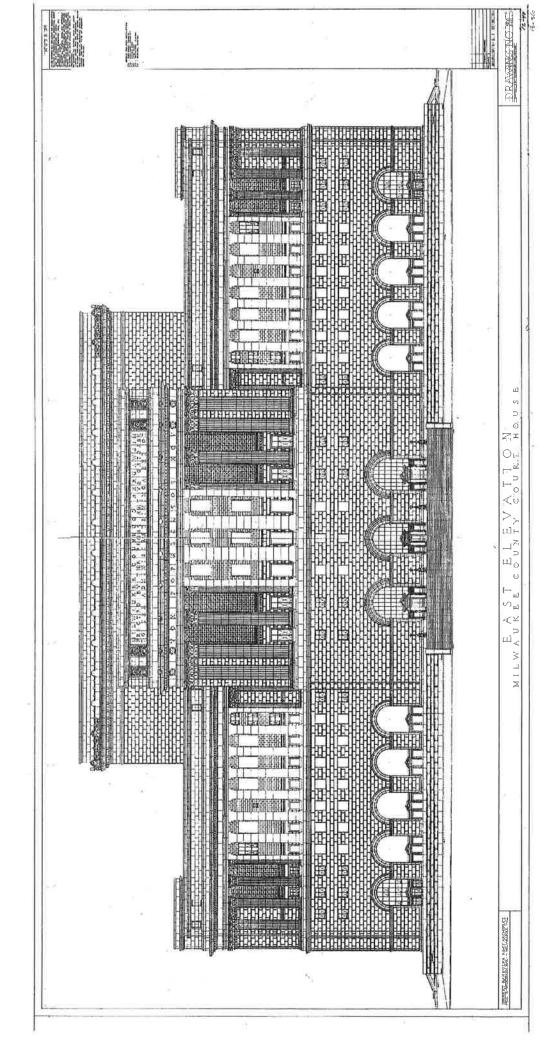
Site Plan Inspection Drop Plan

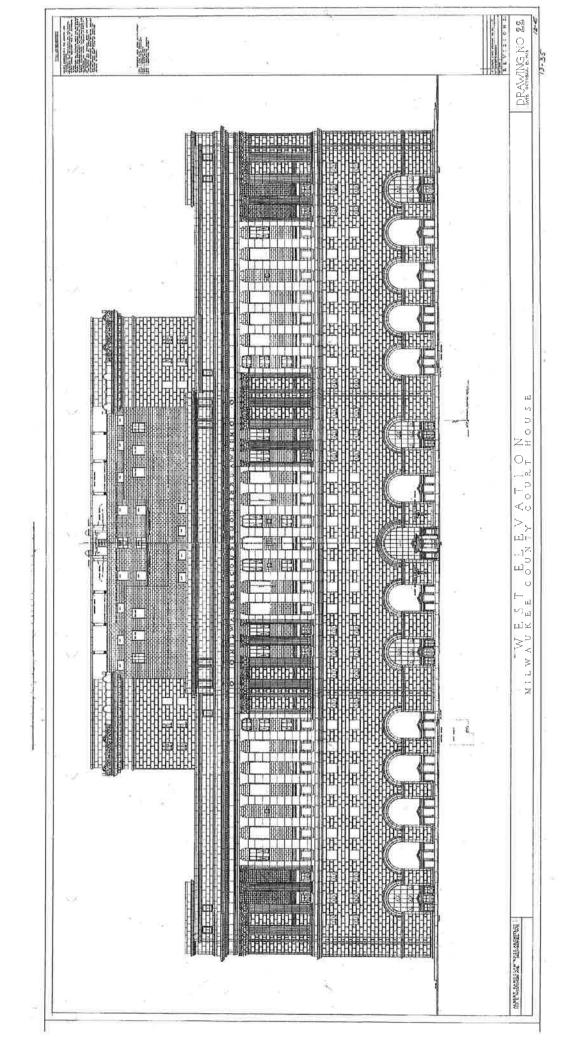


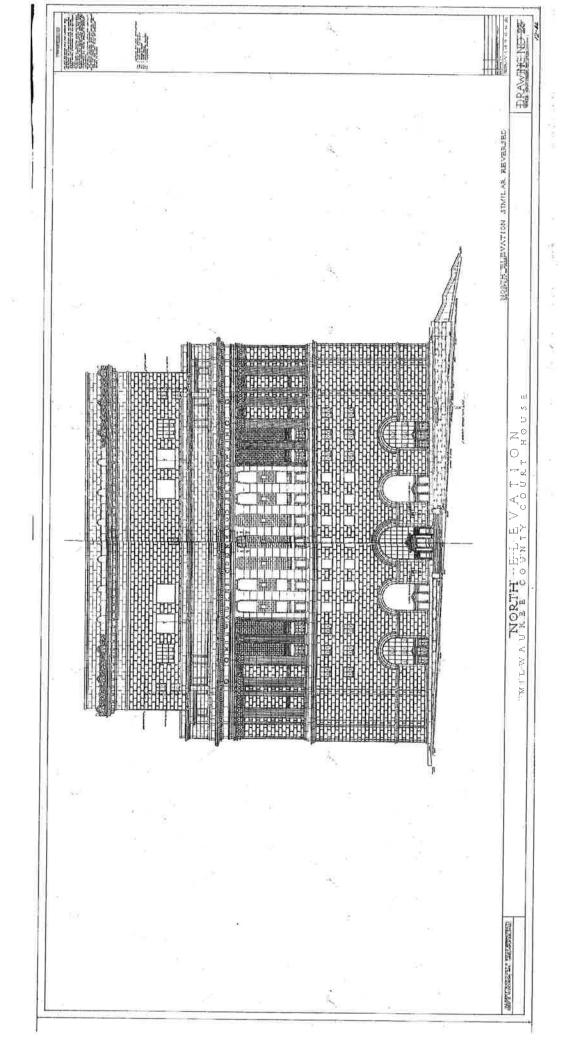


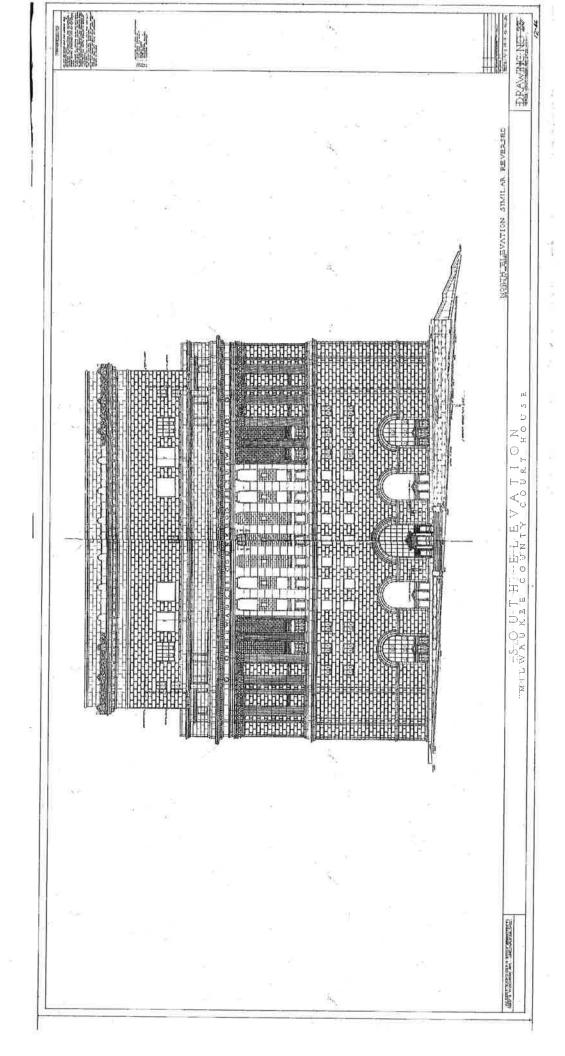
APPENDIX B

Elevation Drawings



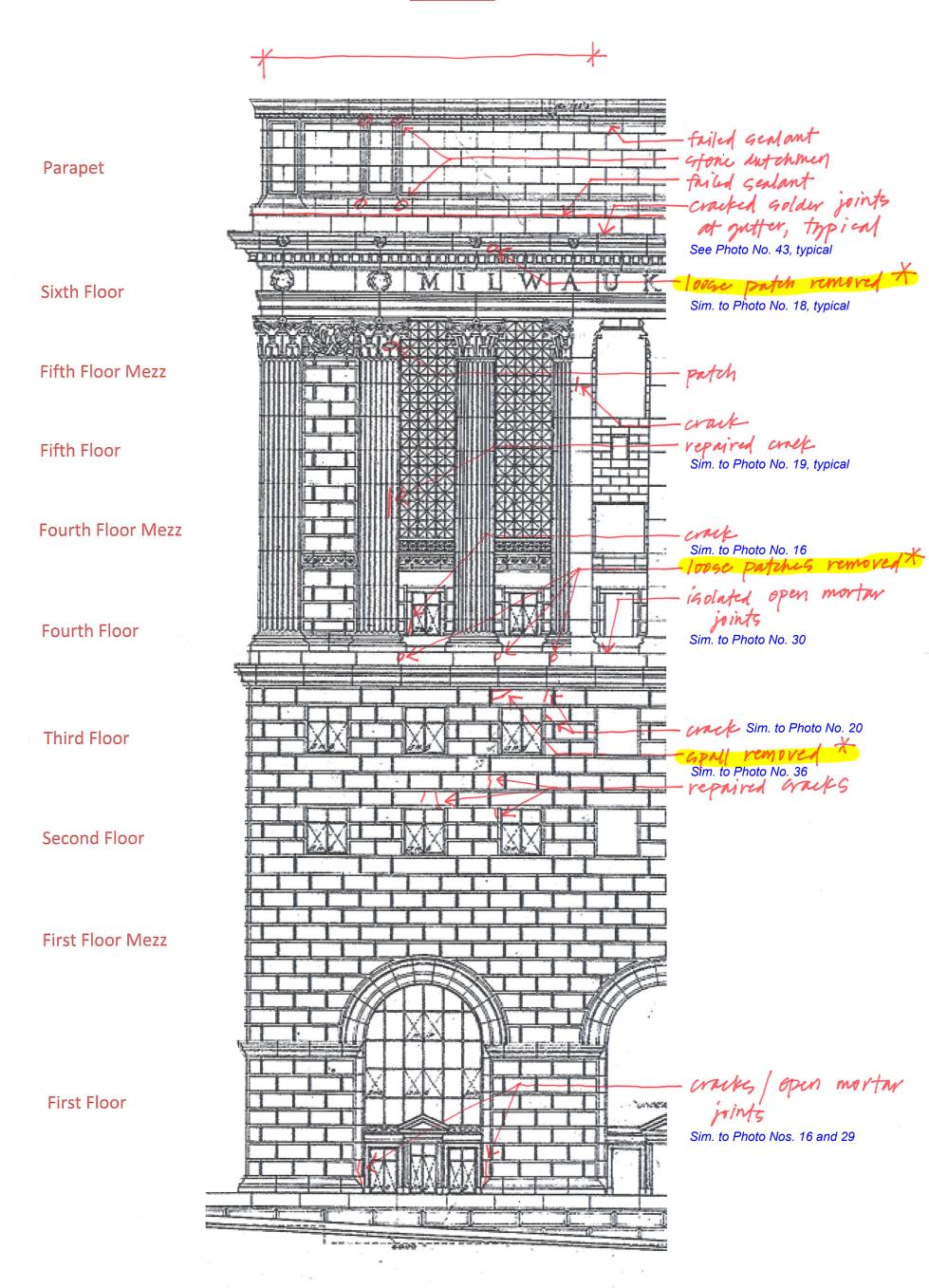




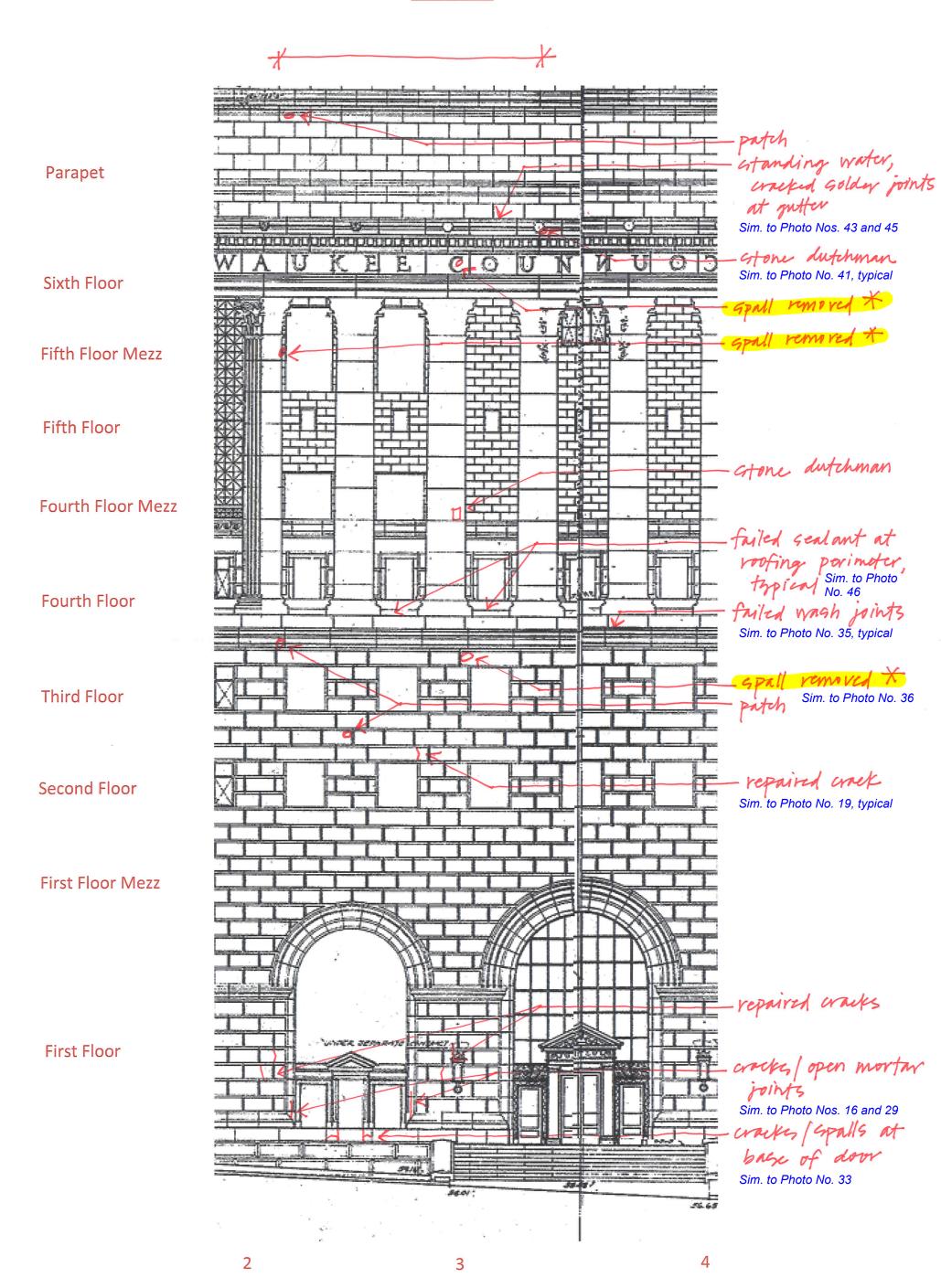


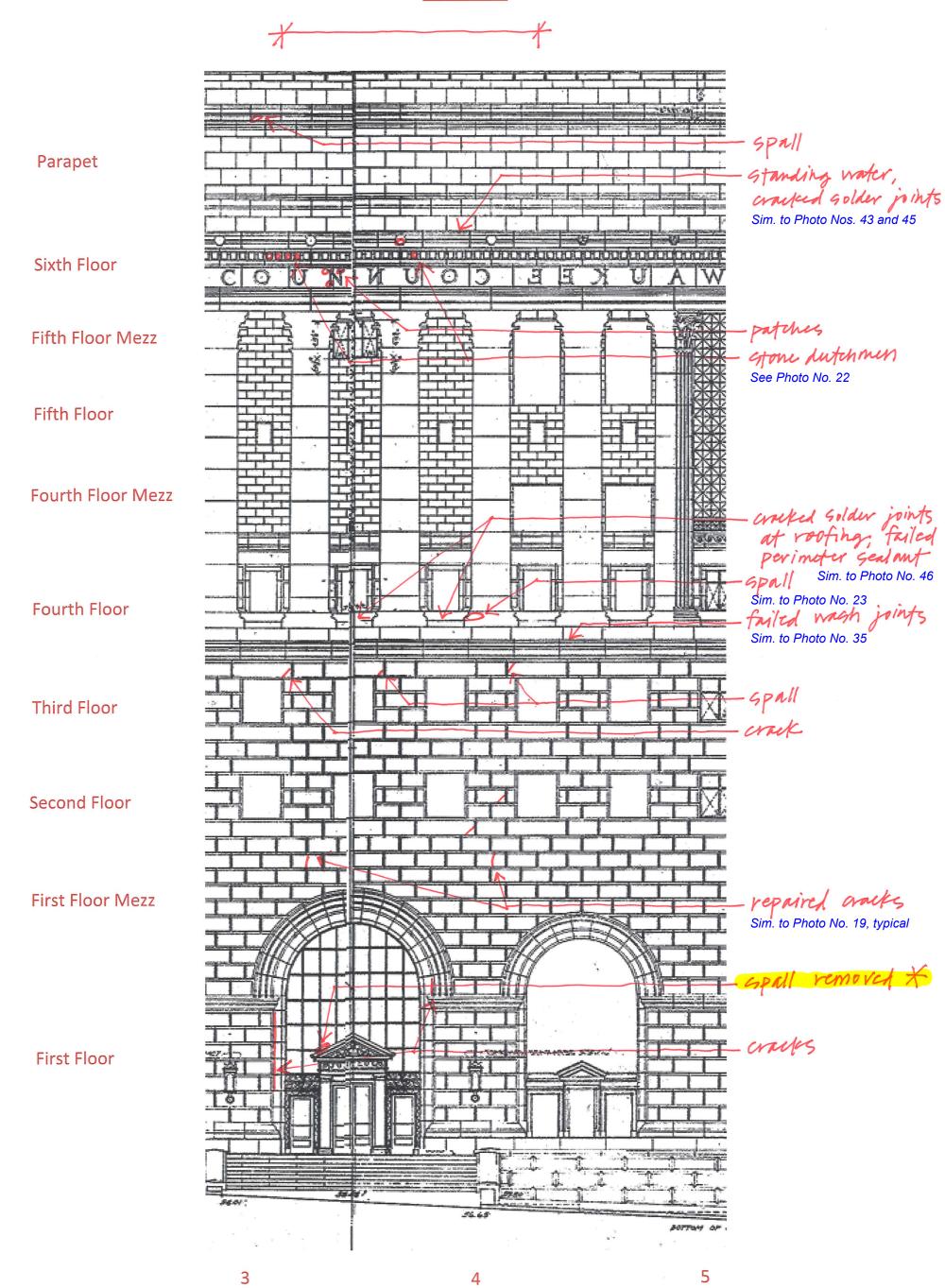
APPENDIX C

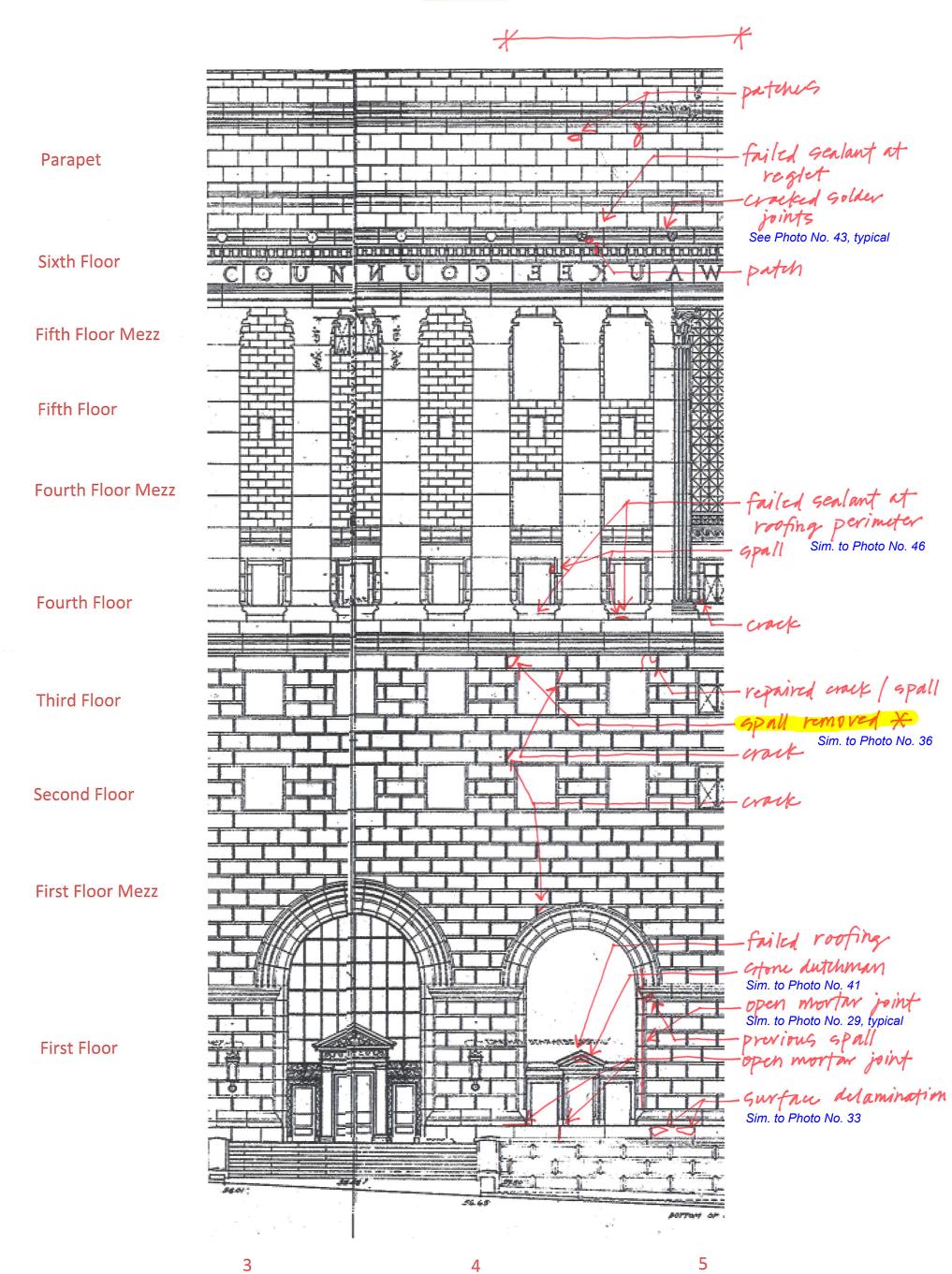
High Reach Lift Survey Sheets Swing stage Scaffold Inspection Drop Sheets

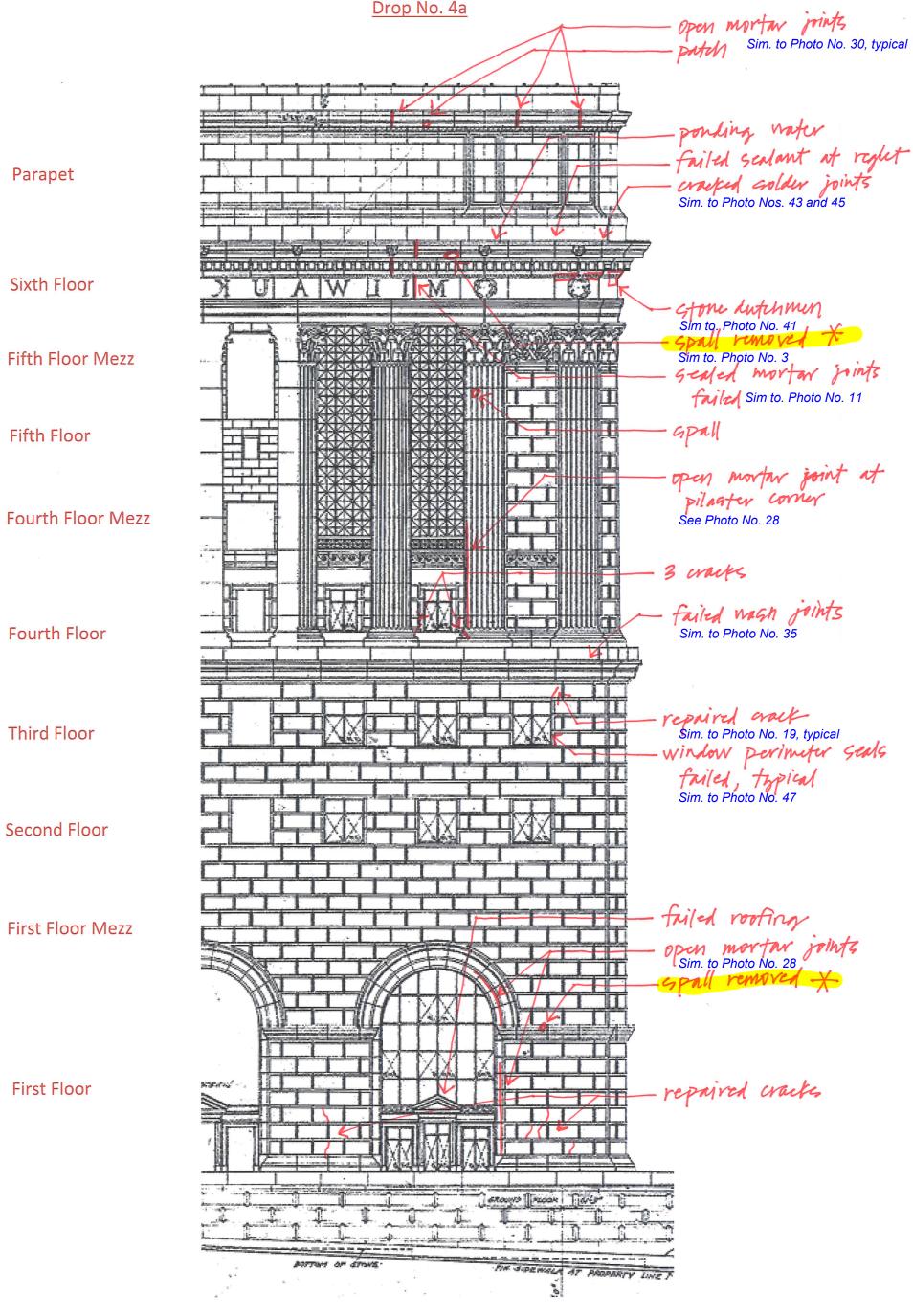


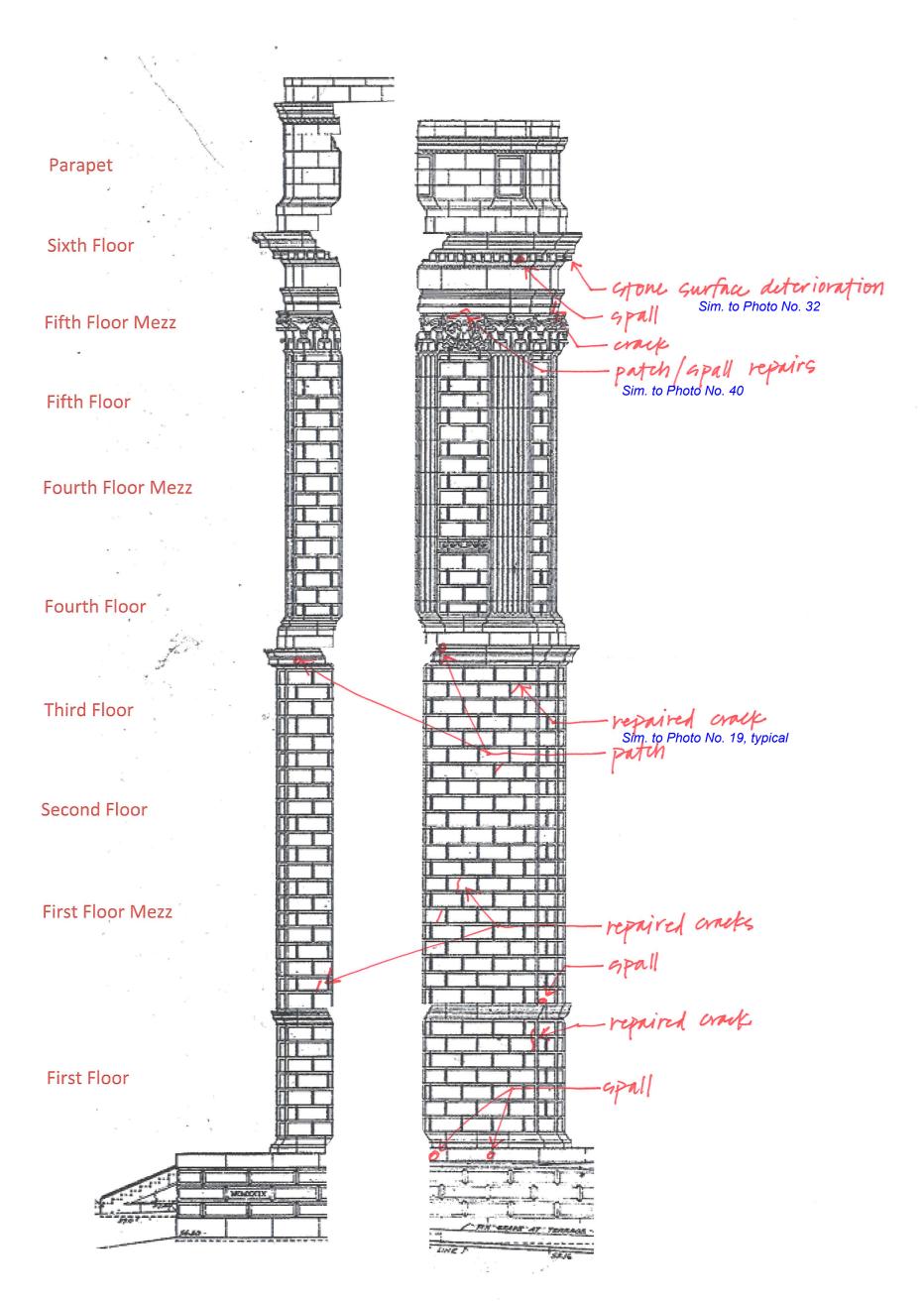
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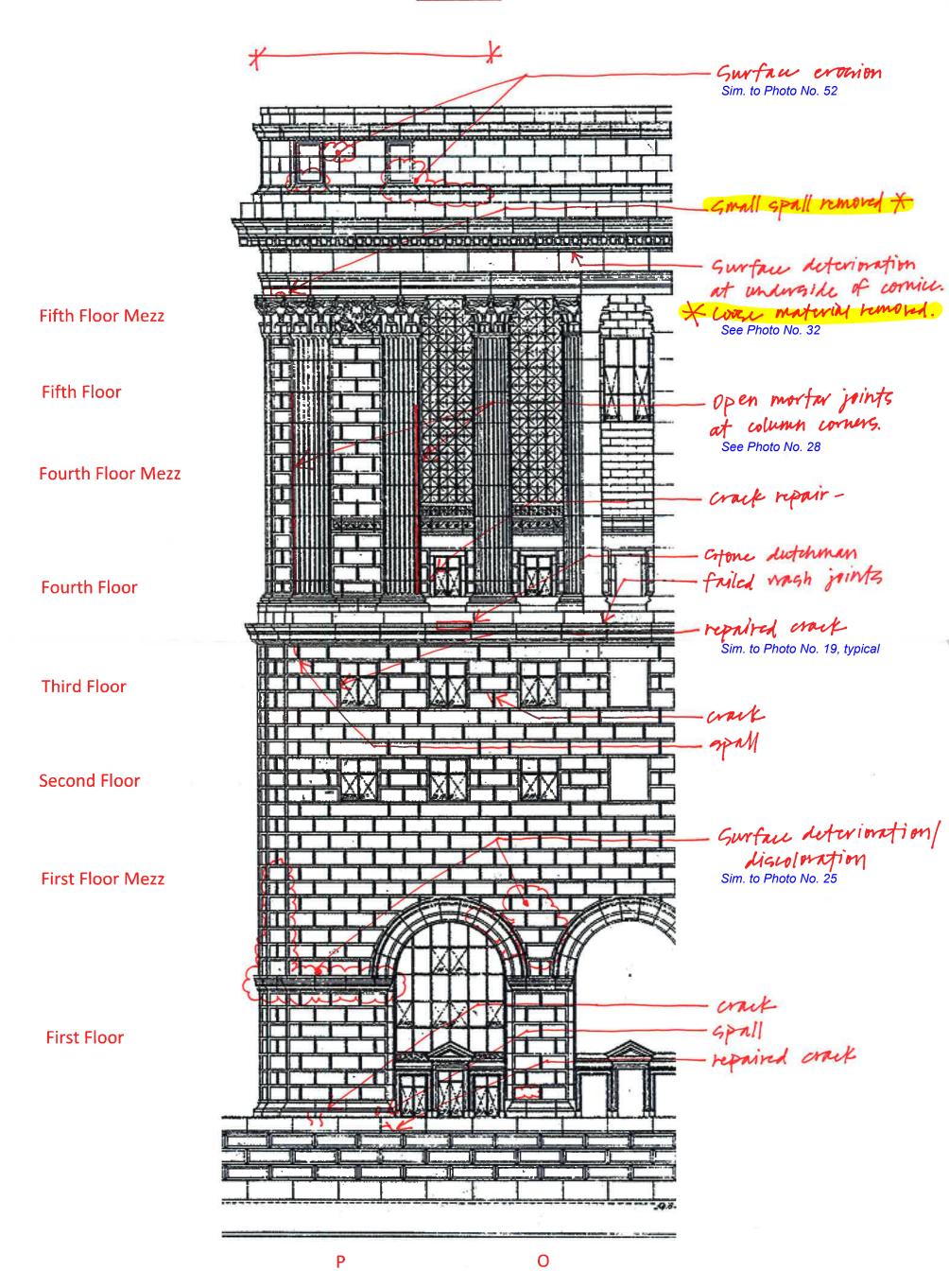






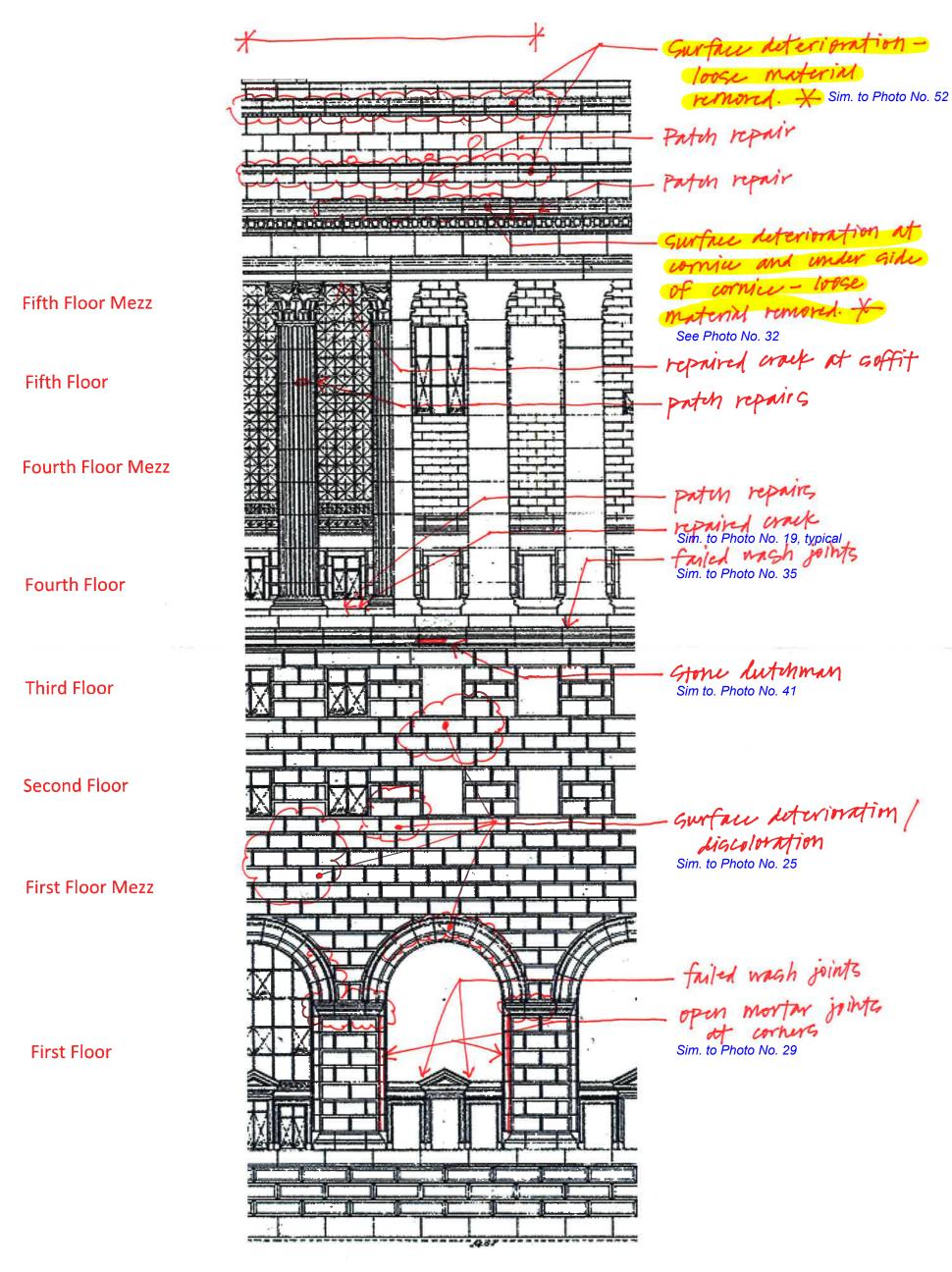




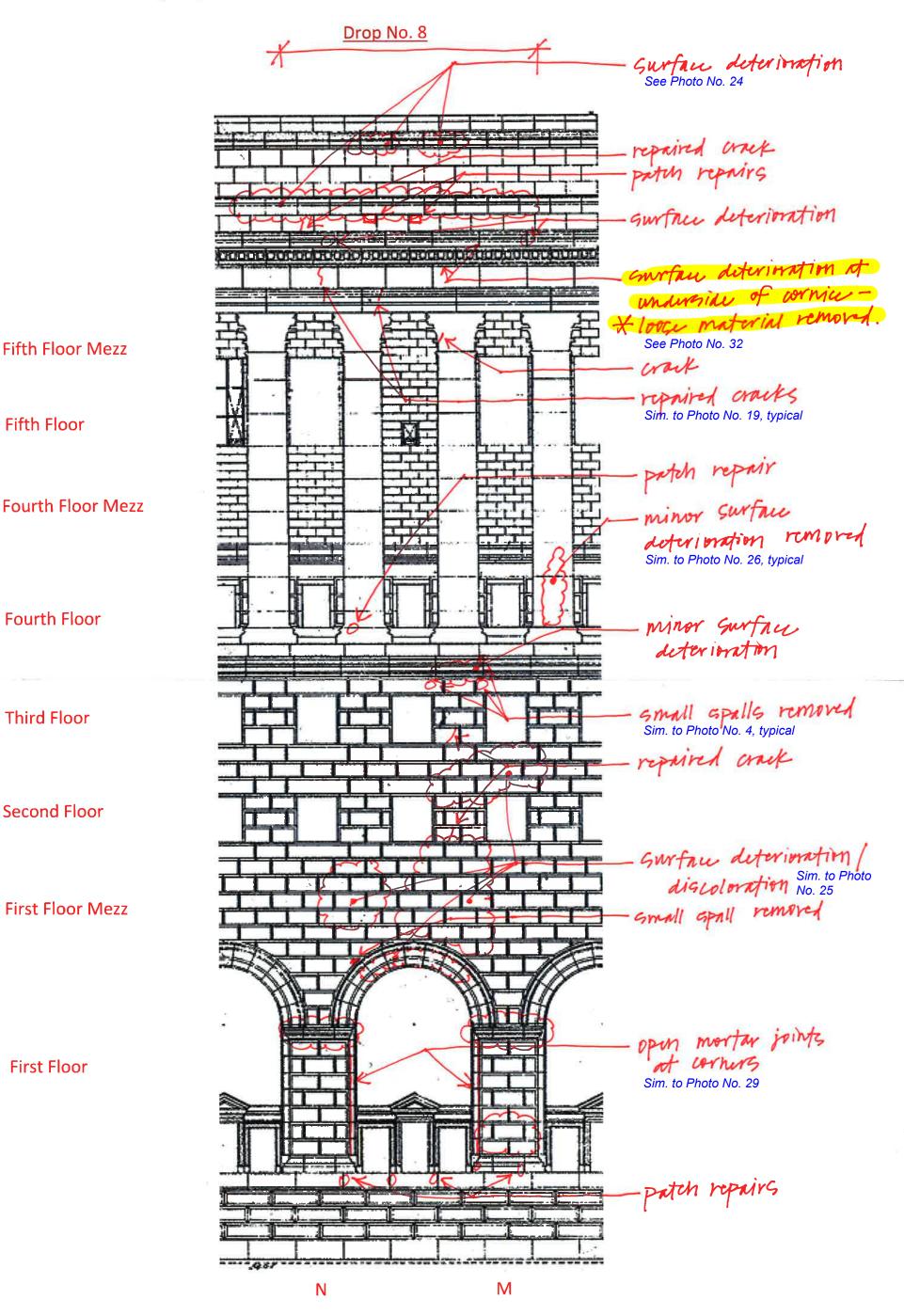


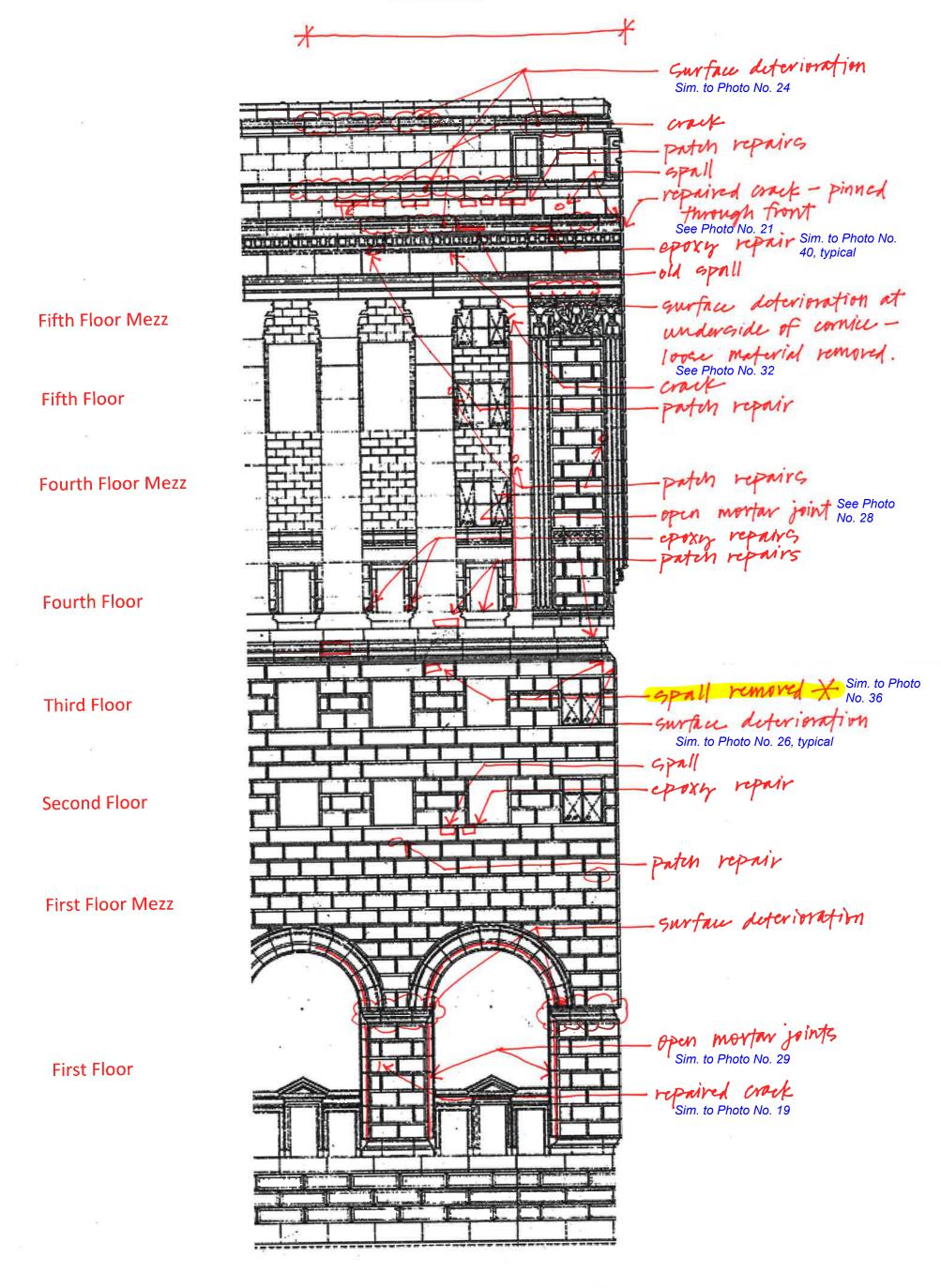
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Drop No. 7



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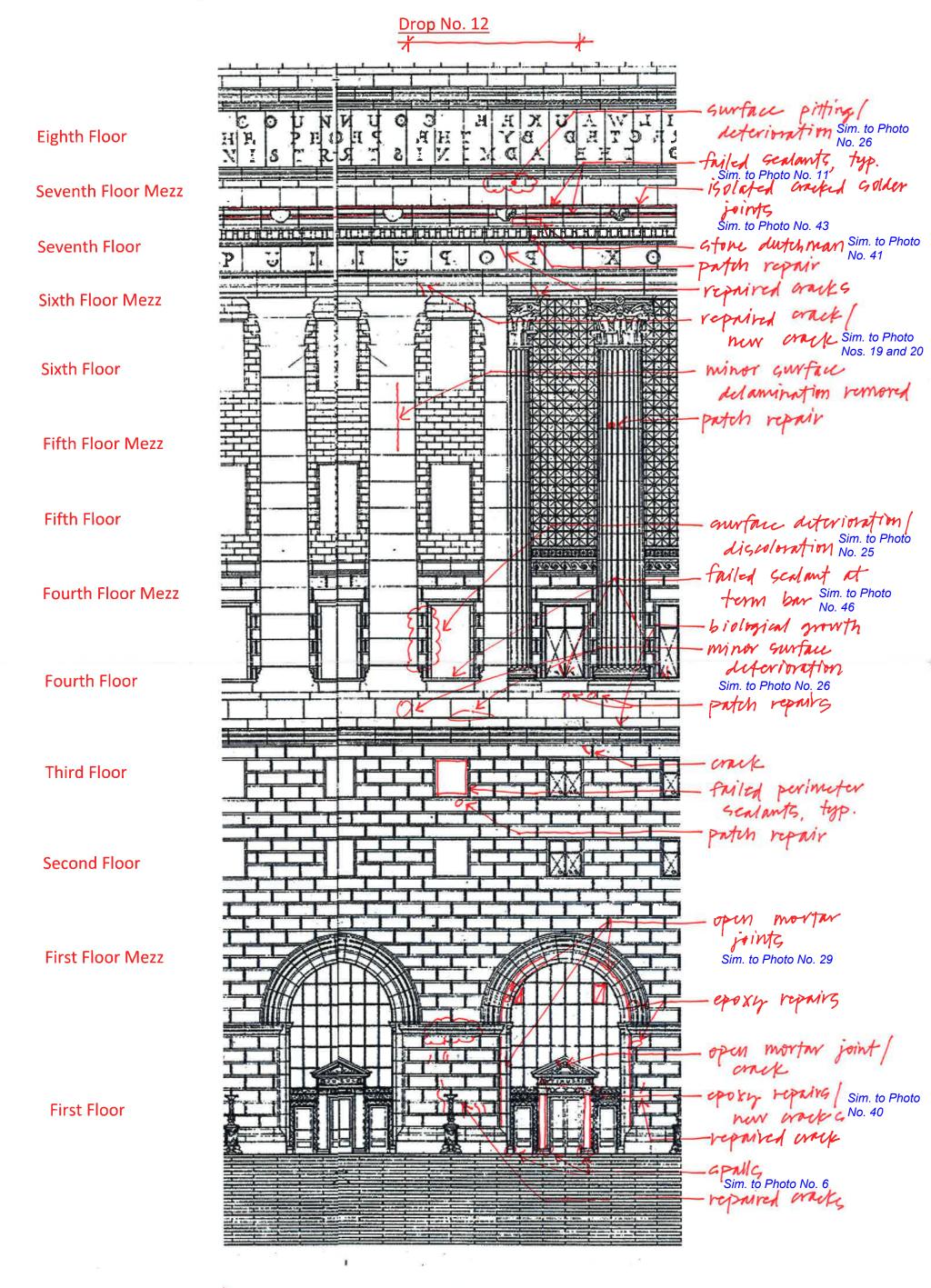


9/2/16 7 = Wigent Repair (Removal Drop No. 11 epoxy repair Sim. to Photo No. 40 KEE MILW FR. Failed Sealants, trp. Sim. to Photo No. 11 **Eighth Floor** Seventh Floor Mezz open mortar pint Sim. to Photo No. 30 0 Seventh Floor patch repair Sixth Floor Mezz cracks - minor cofone delamination Sim. to Photo No. 26, typical Sixth Floor crack repair Sim. to Photo No. 19, typical Fifth Floor Mezz failed seplant of tem bow Sim. to Photo No. 46 Fifth Floor patch repair -old apall Fourth Floor Mezz minor surface deterioration Sim. to Photo No. 26, typical Fourth Floor aurface pitting putch repair failed perimeter Genlants, Third Floor Sim. to Photo No. 47 repaired once Second Floor minor surface deterioration First Floor Mezz epoxy repair See Photo No. 40 open mortar joints at cornus Sim. to Photo No. 29 Spalls Sim. to Photo No. 33 loose patch material First Floor

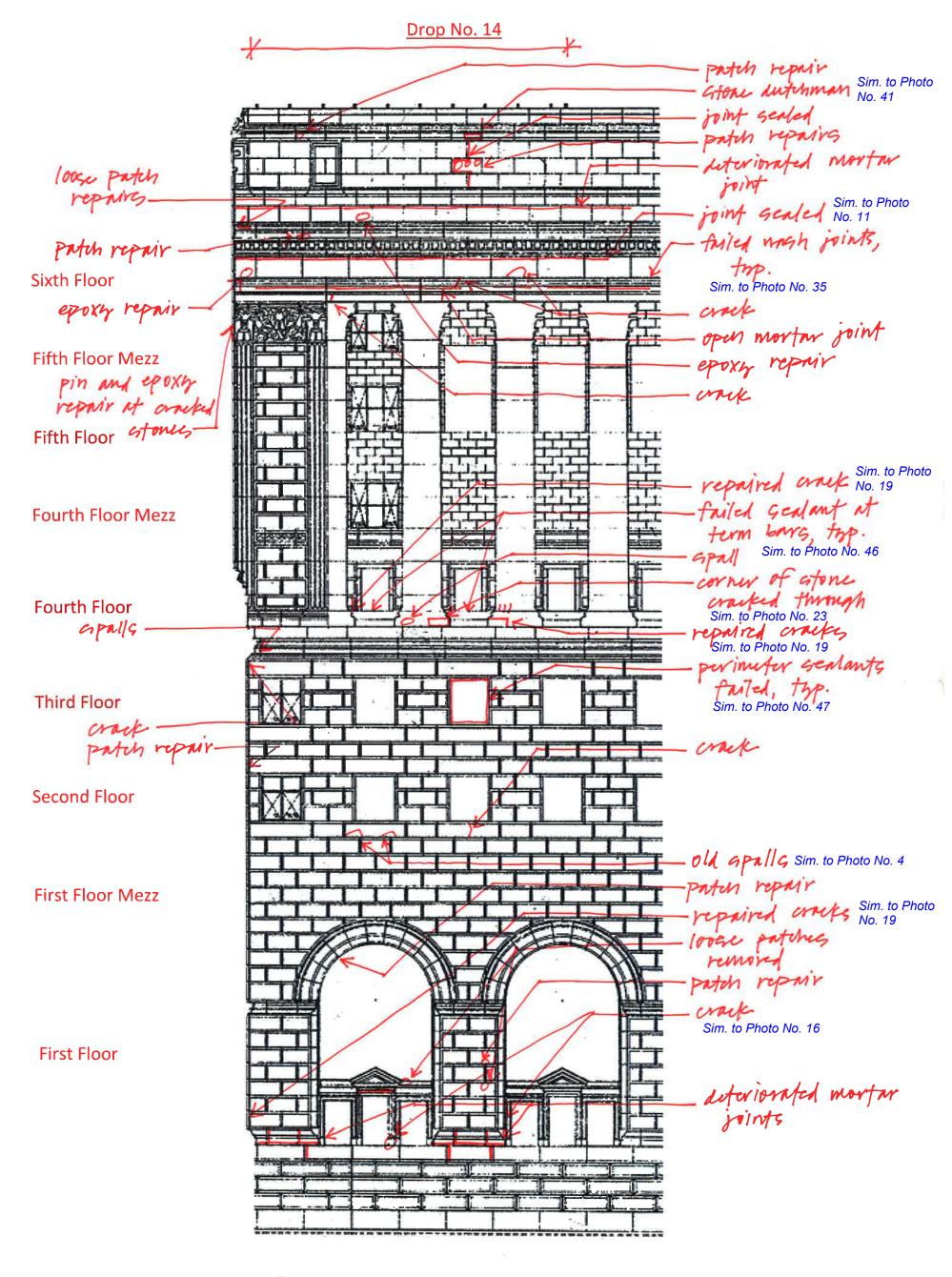
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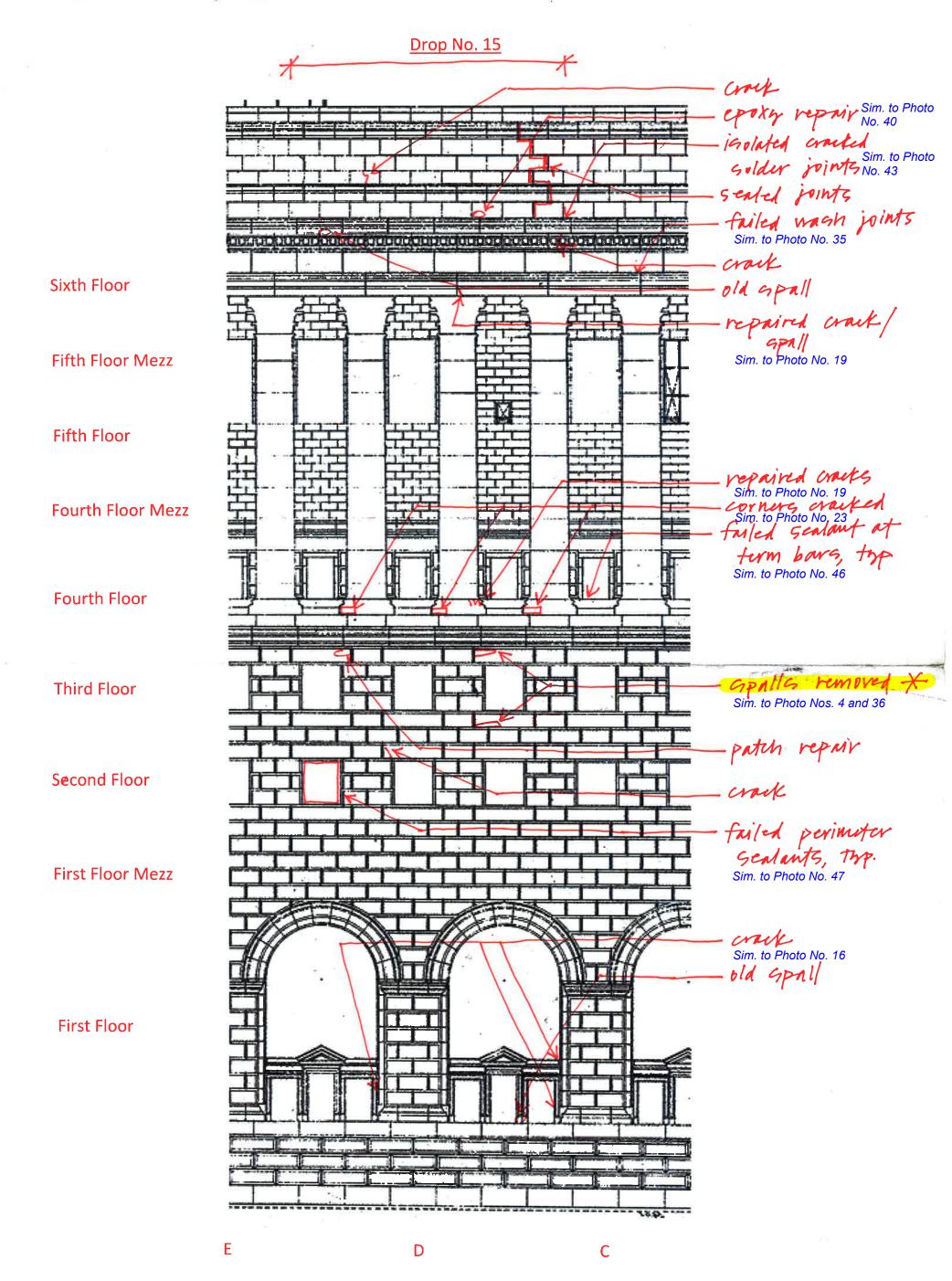
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Drop No. 11a - minor Gurface delamination AUXEL CO ED BY THE E ADM NIS Eighth Floor Gone autchmen Sim. to Photo No. 41 Surface deterioration Seventh Floor Mezz Sim. to Photo No. 26, typical Seventh Floor efflouresana required aracks Sim. to Photo No. 19, typical Sixth Floor Mezz Sixth Floor Fifth Floor Mezz minor surface delamination removed Fifth Floor - cost -failed sealant at Fourth Floor Mezz term bur Sim. to Photo No. 46 - 1 ange GPA // Sim. to Photo No. 23 -biological growth **Fourth Floor** minor surface paters repairs repaired cracks **Third Floor** failed perimeter Gealants, typ. Sim. to Photo No. 47 Second Floor Surface deterioration/ discoloration. Sim. to Photo No. 25 First Floor Mezz See Photo No. 40 revalued crack First Floor Surface deterioration Sim. to Photo No. 33 SPALL Sim. to Photo No. 6

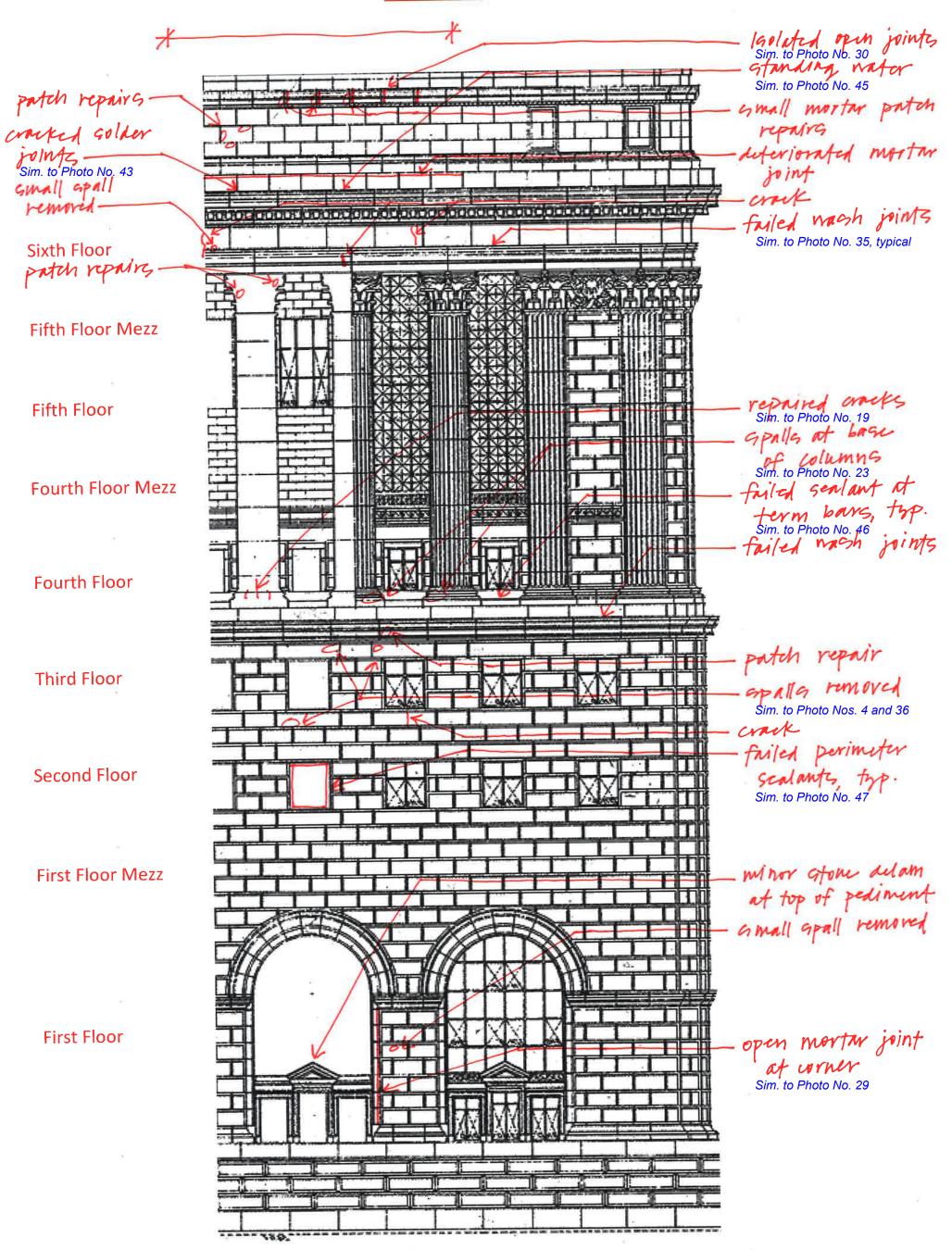


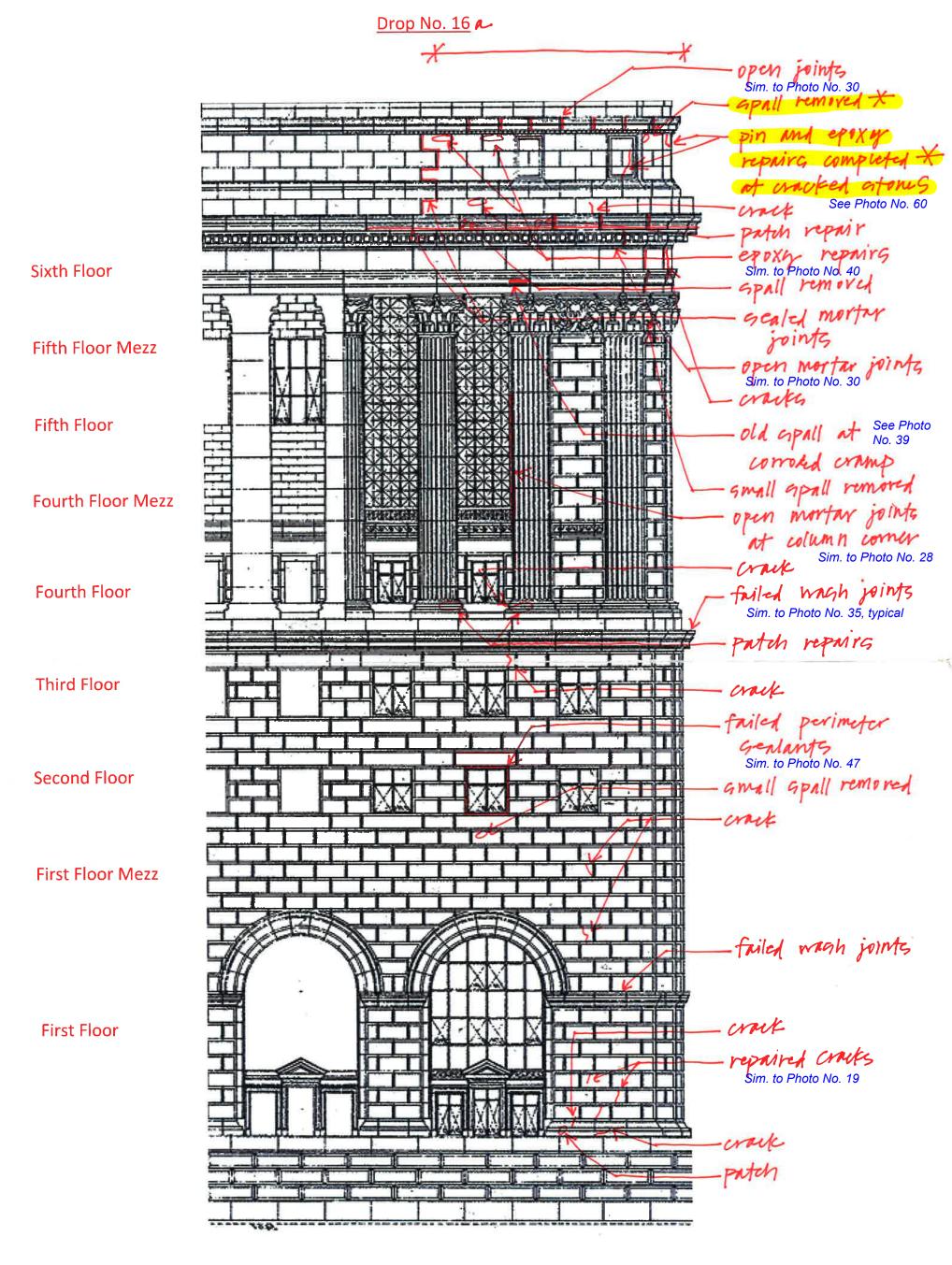
Drop No. 13 patch repair pitting / surface deterioration Sim, to Photo No. 26 failed sealants, typ. Sim. to Photo No. 11 **Eighth Floor** FR. G A Seventh Floor Mezz failed wash joints Sim. to Photo No. 35 open mortar joints Sim. to Photo No. 30 Seventh Floor paten repairs Sixth Floor Mezz Sim. to Photo No. 19 small spall removed Sixth Floor crack Pin and cpoxy Sim. to Photo No. 21 Fifth Floor Mezz Open mortar joint Sim. to Photo No. 28 Fifth Floor epoxy repair Sim. to Photo No. 40 - large crack Sim. to Photo No. 23 repaired cracks Sim. to Photo Porten repair Fourth Floor Mezz **Fourth Floor** open morter joint Sim. to Photo No. 30 repaired ancks (Sim. to Photo new wacks Nos. 19 and 20 Third Floor april **Second Floor** surface deterioration / discoloration Sim. to Photo No. 25 - patch repair / spall - cracks First Floor Mezz surface deterioration Sim. to Photo No. 26 First Floor



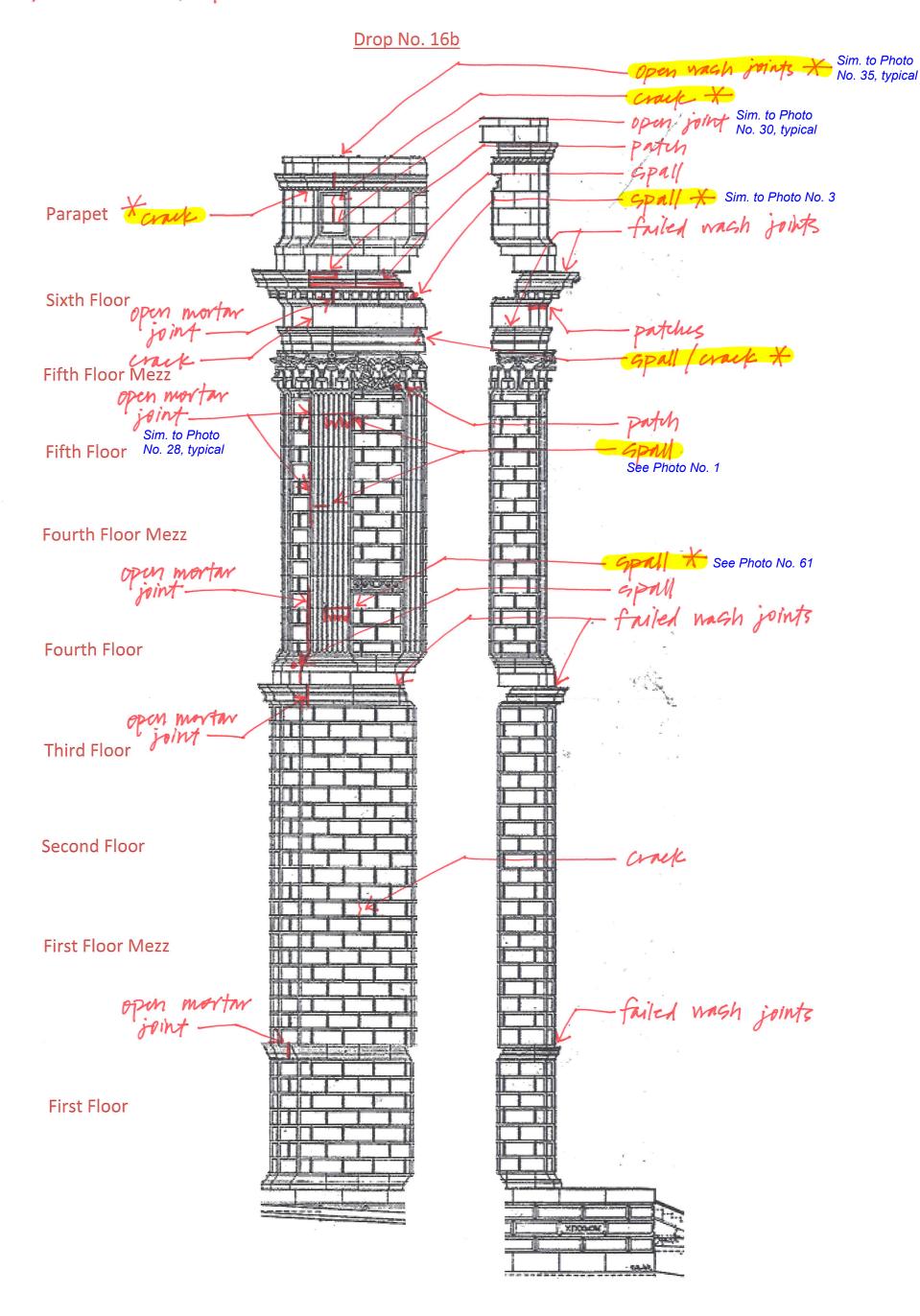


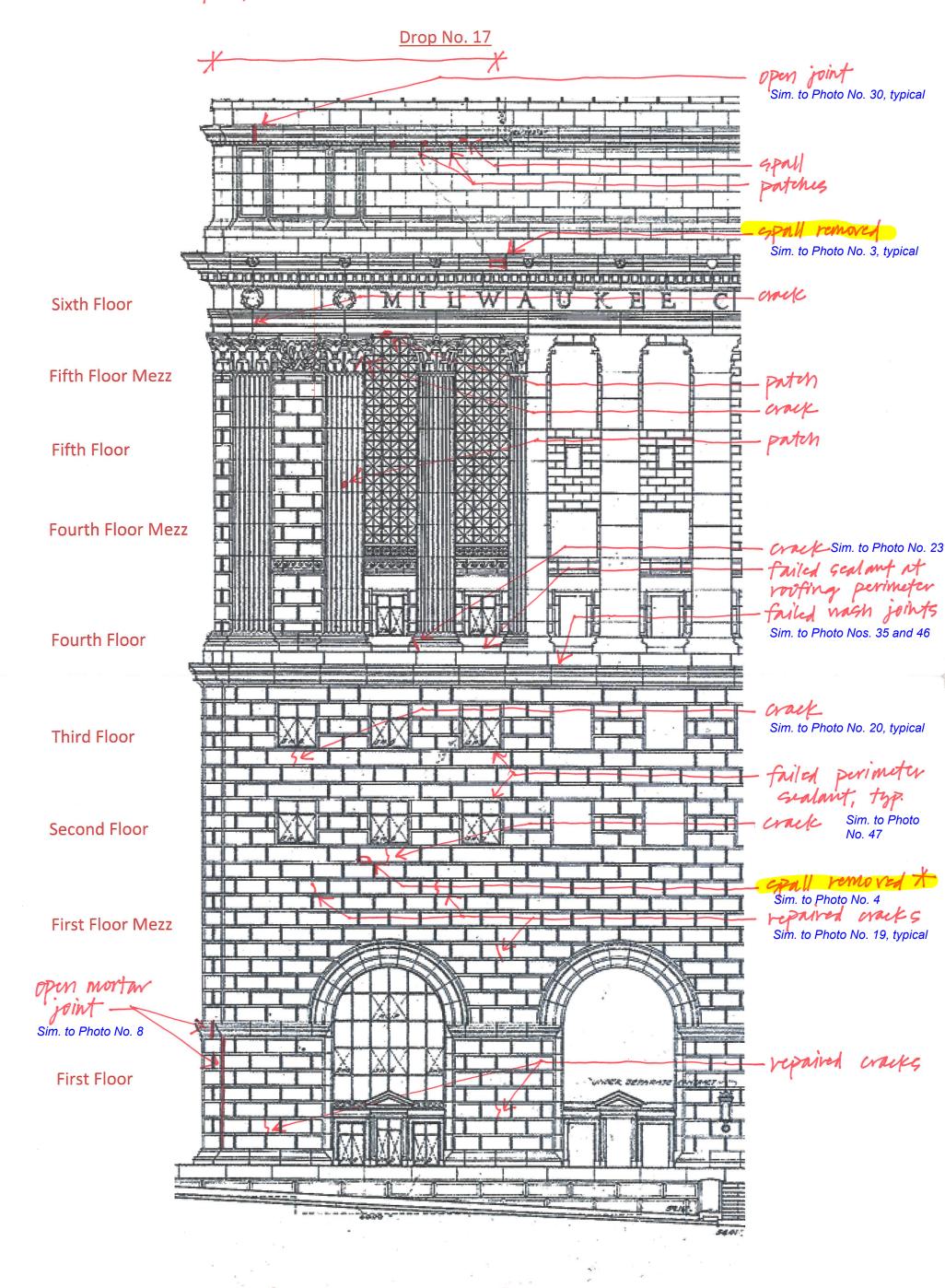
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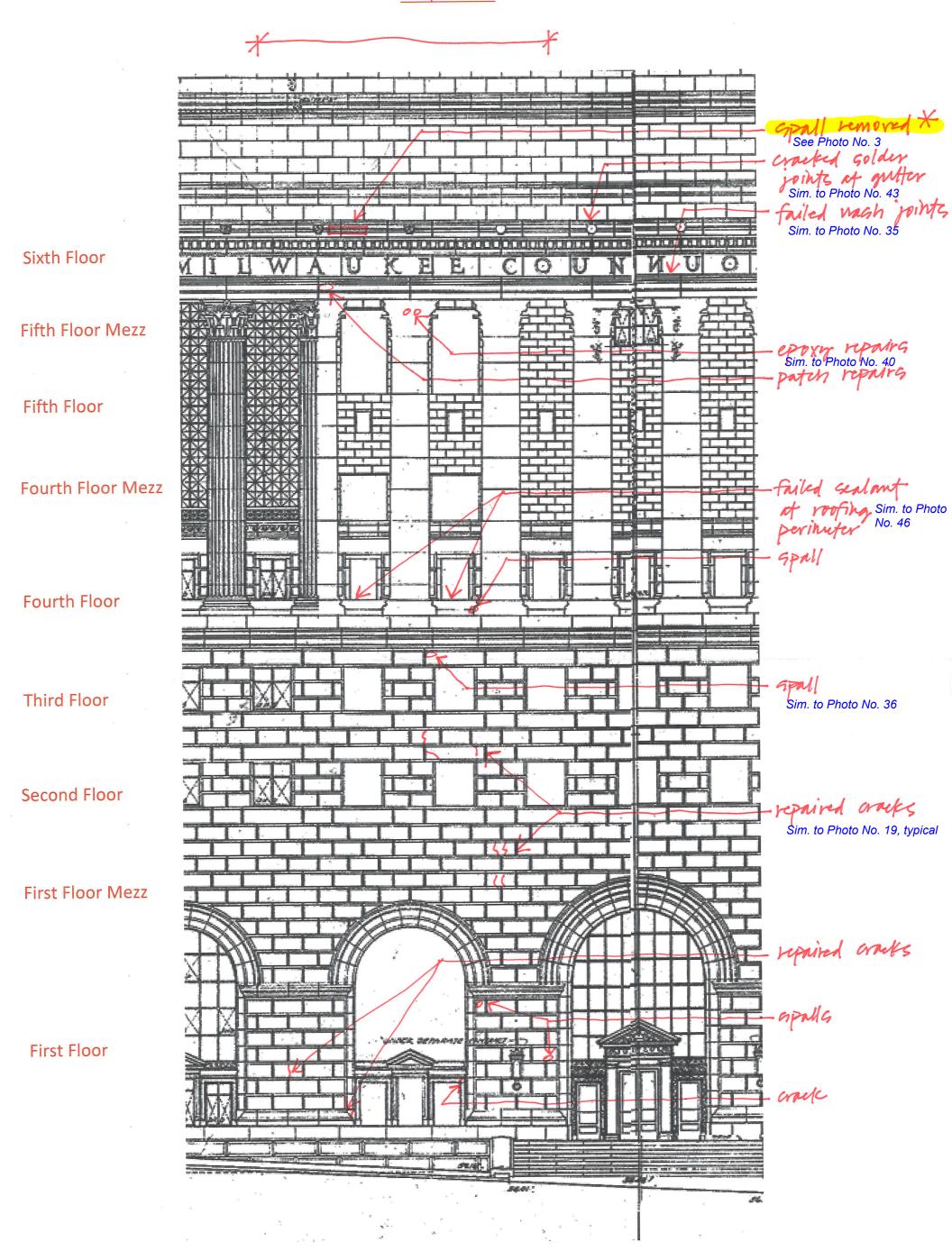




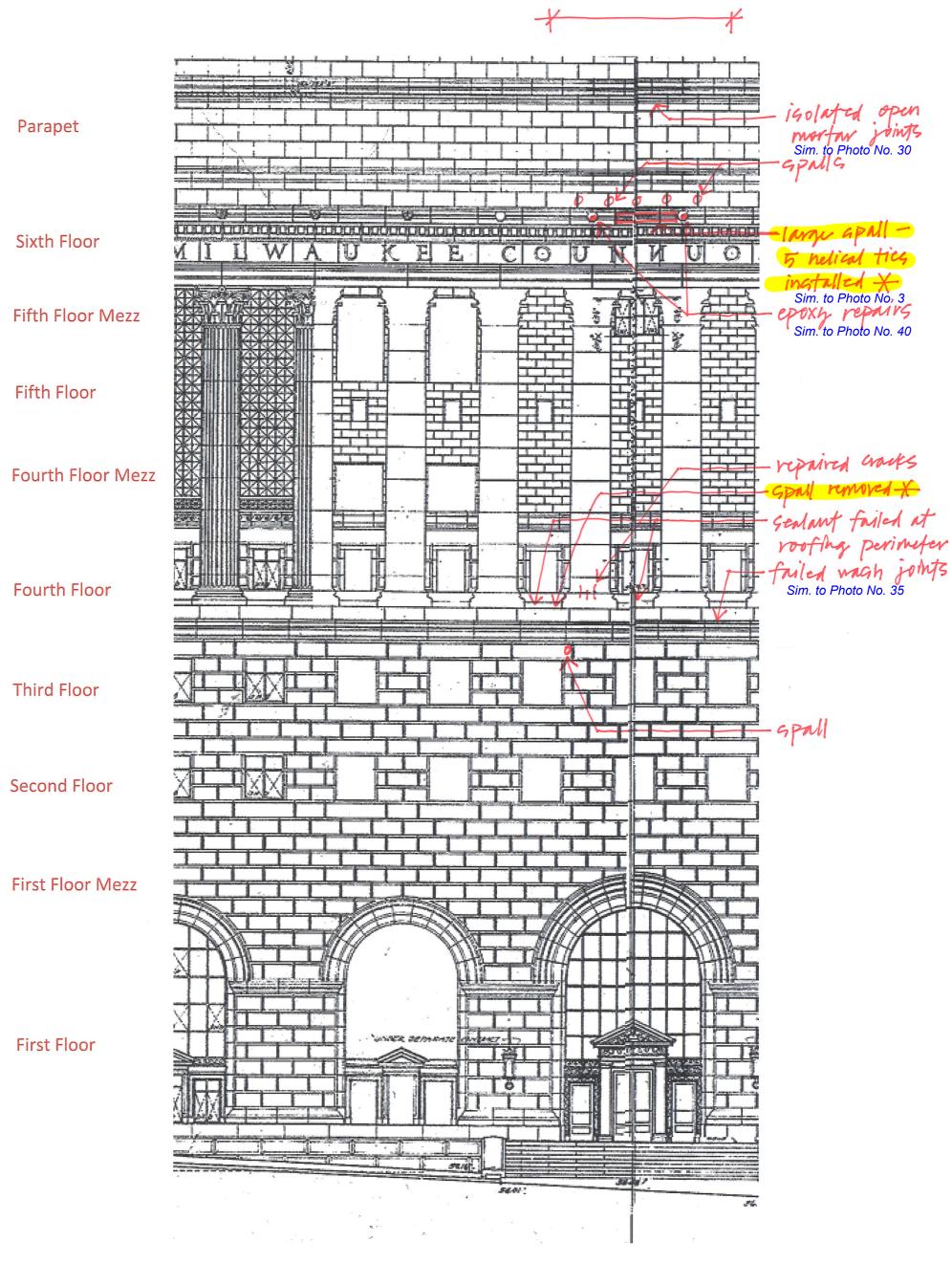
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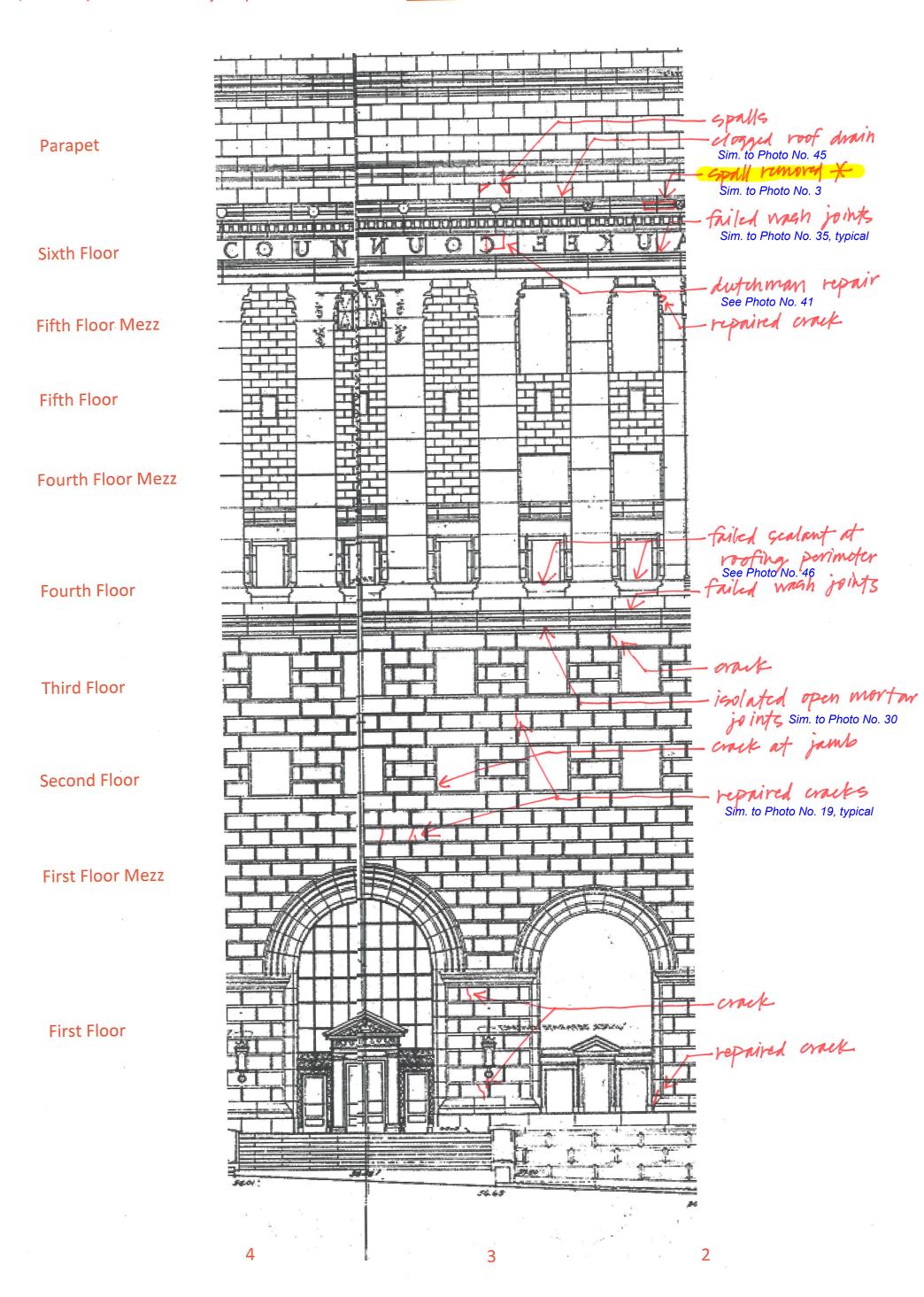






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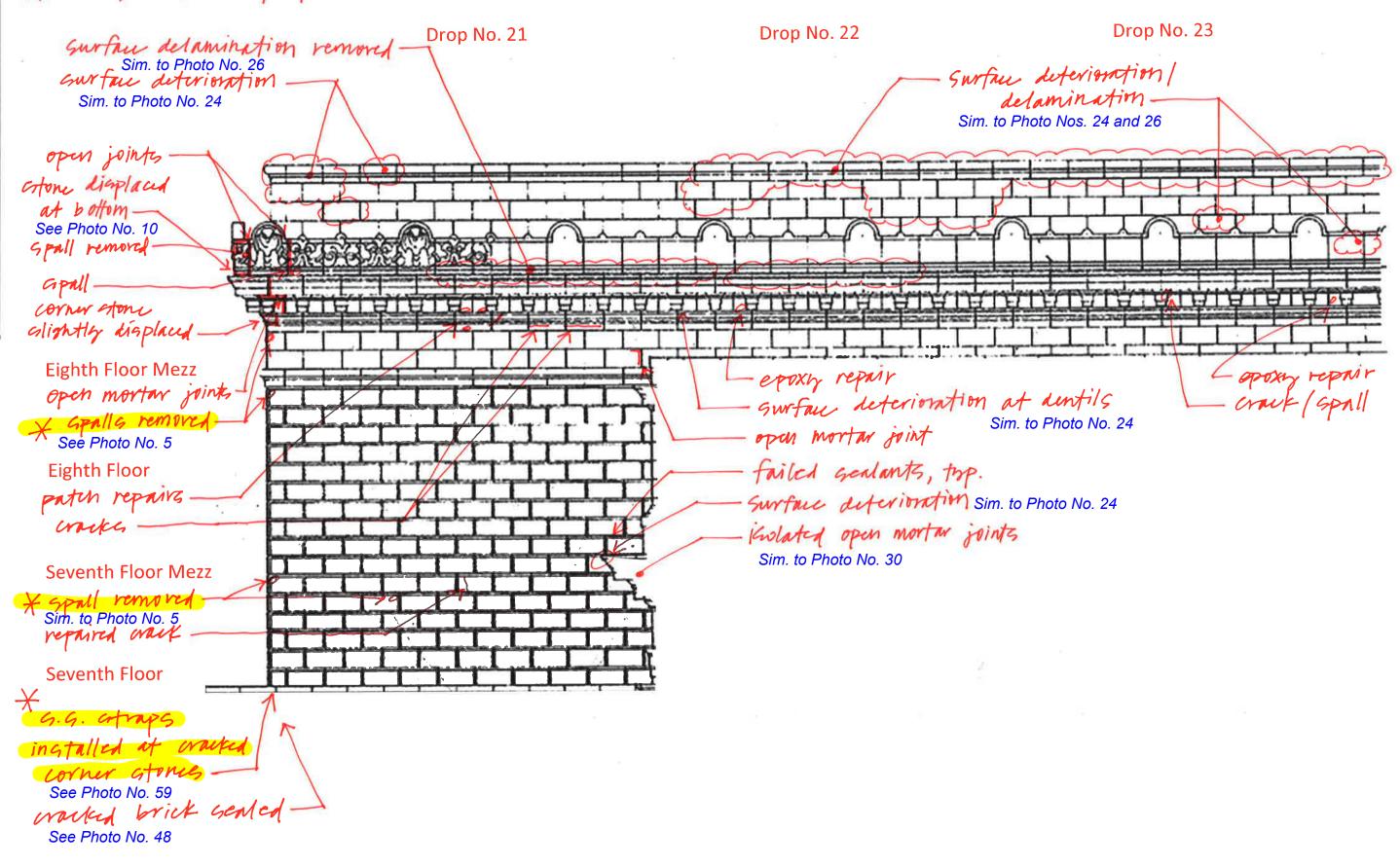


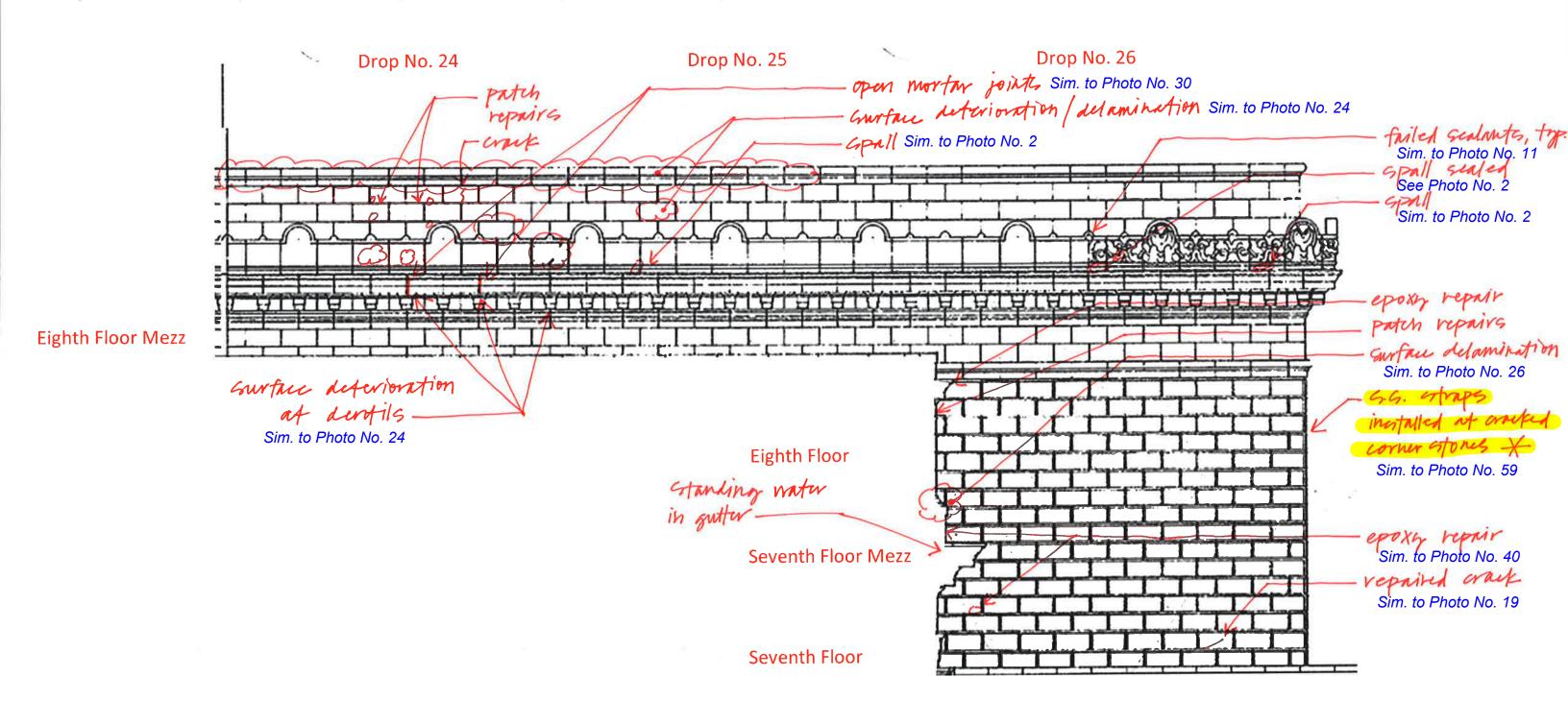


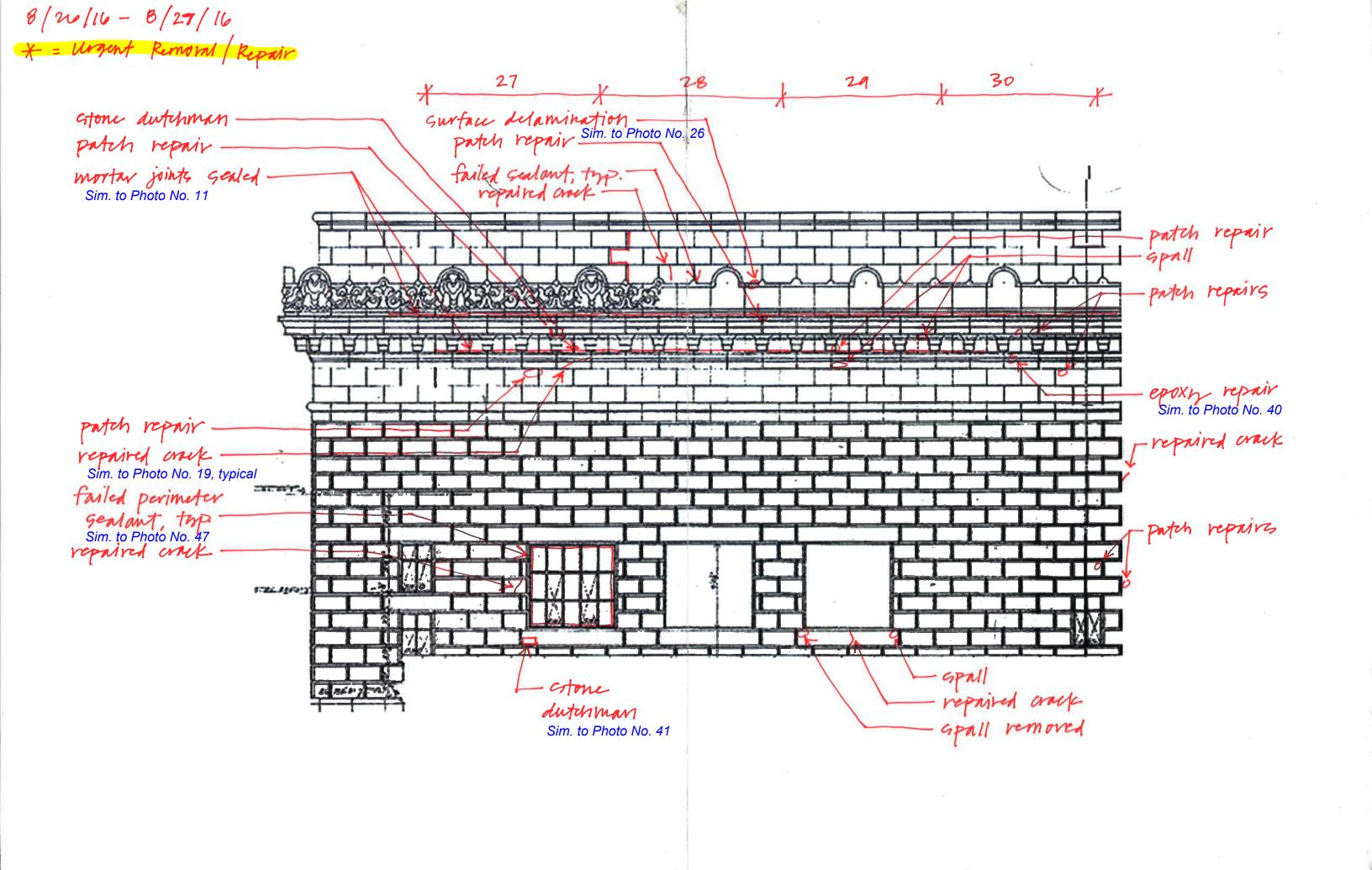
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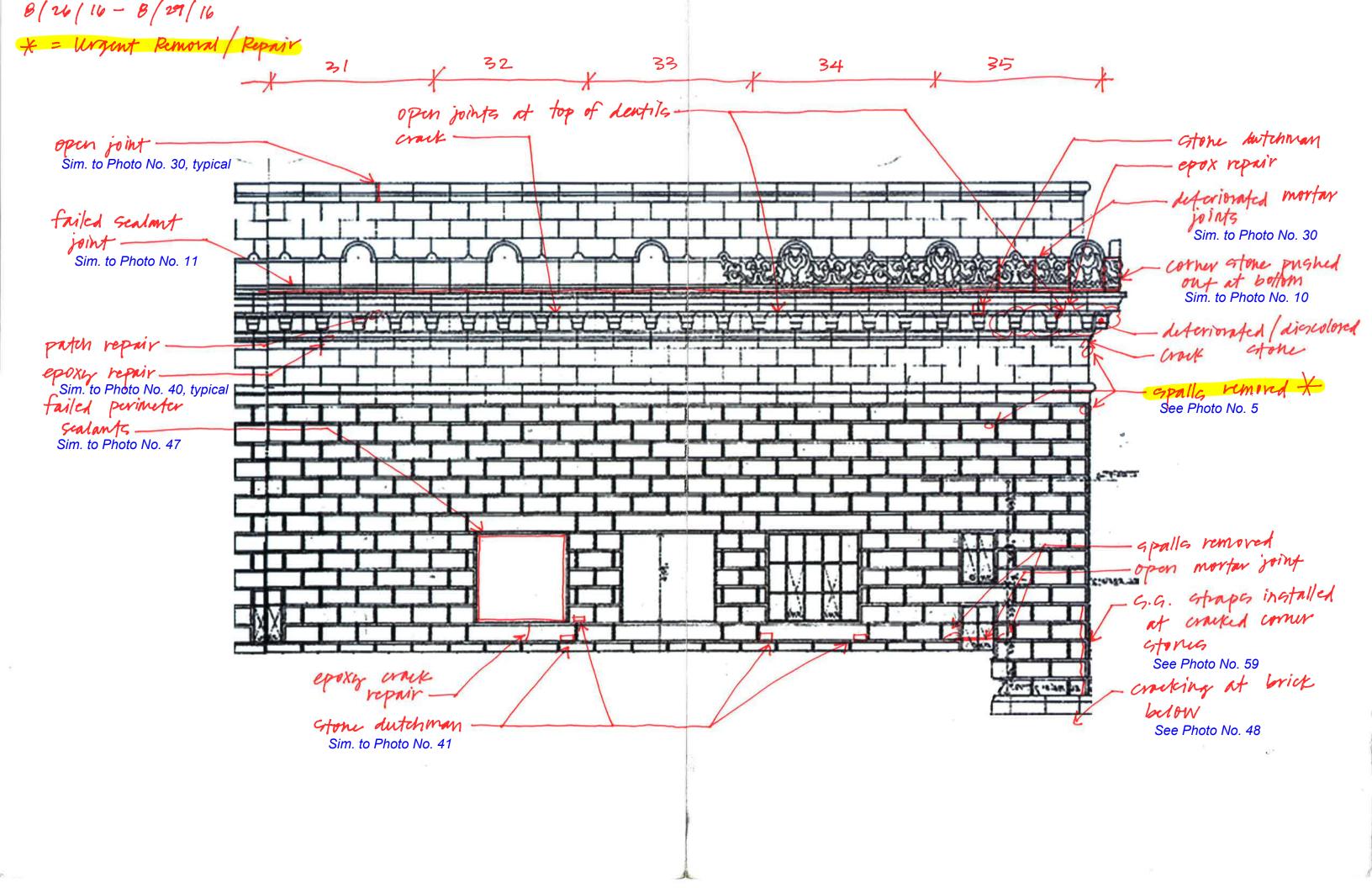
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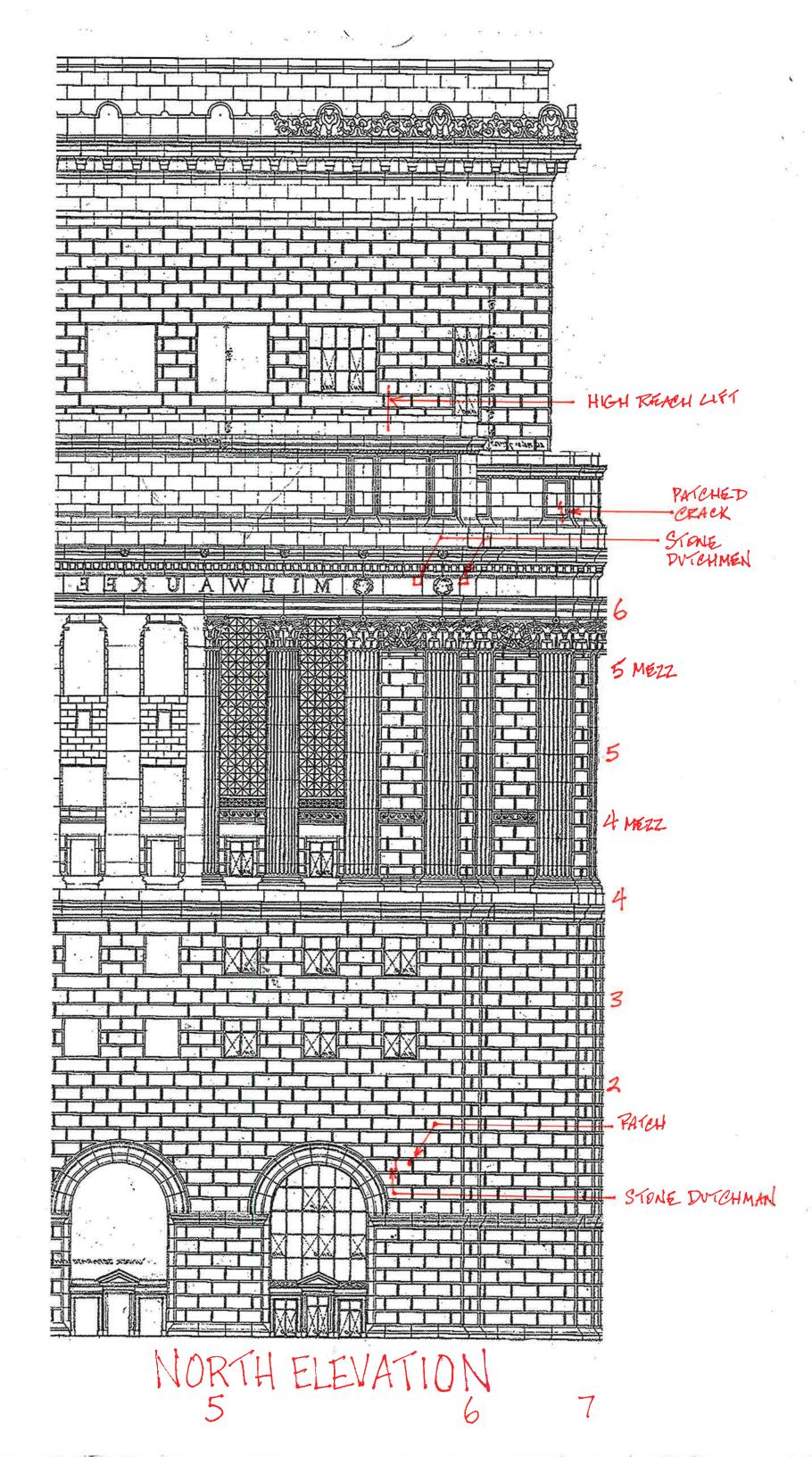
8/25/16, 8/31/16 X = Urgent Removal/Repair

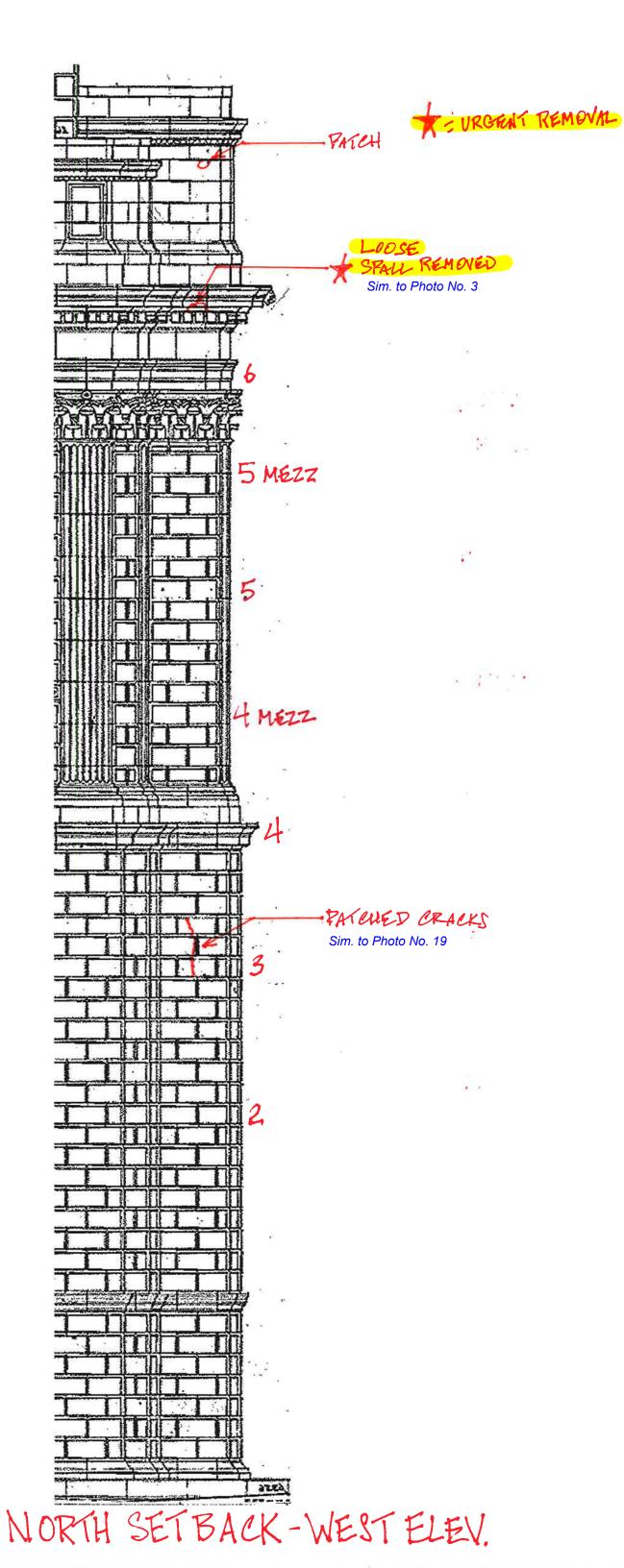




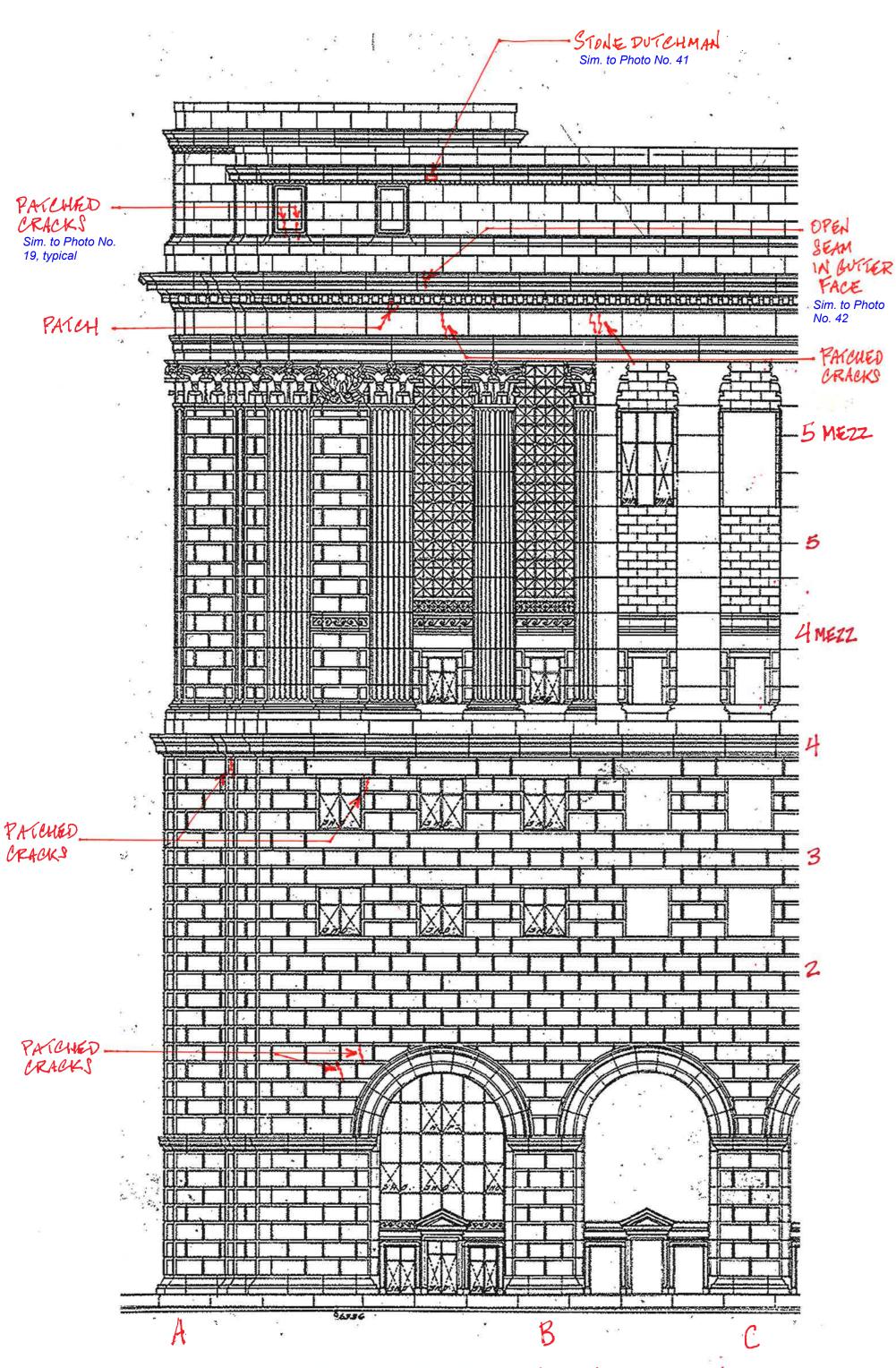




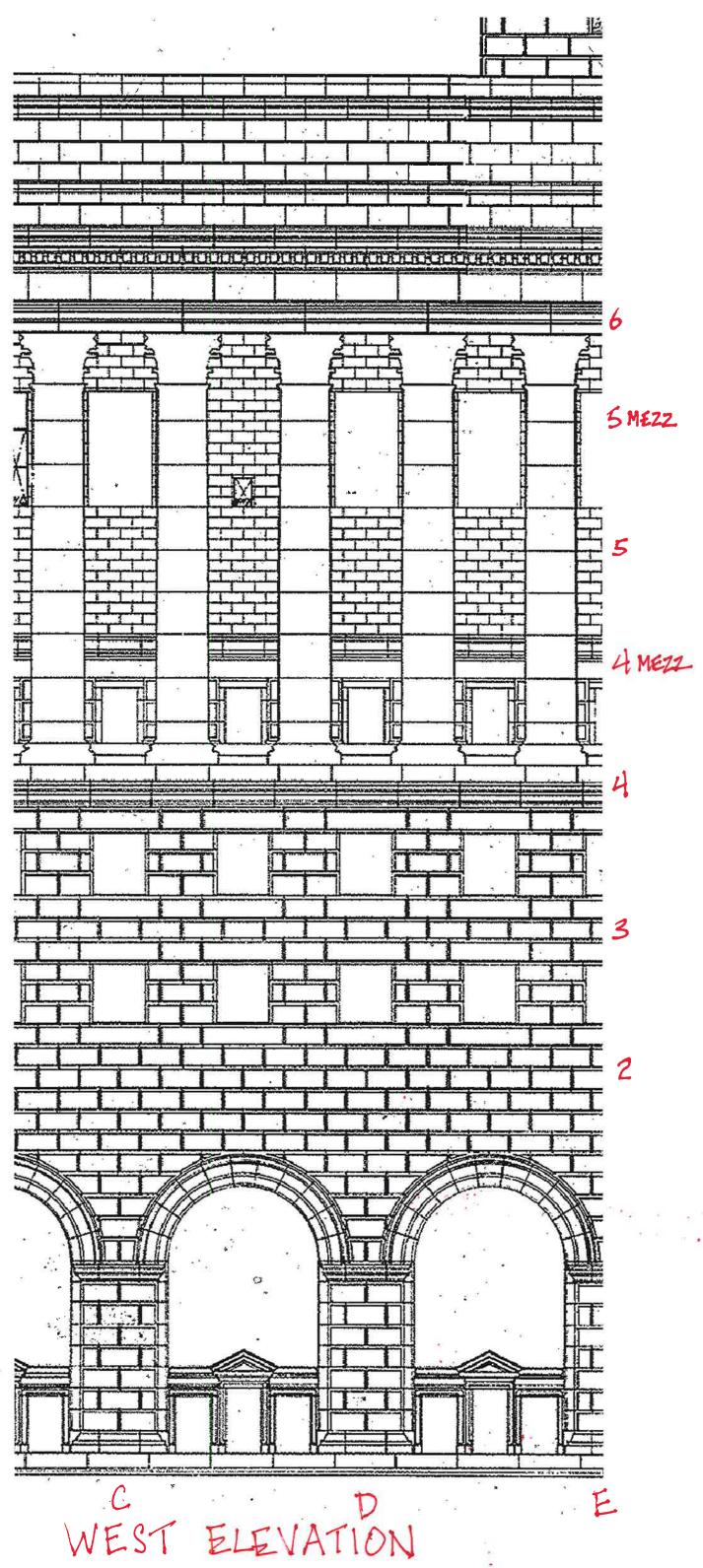


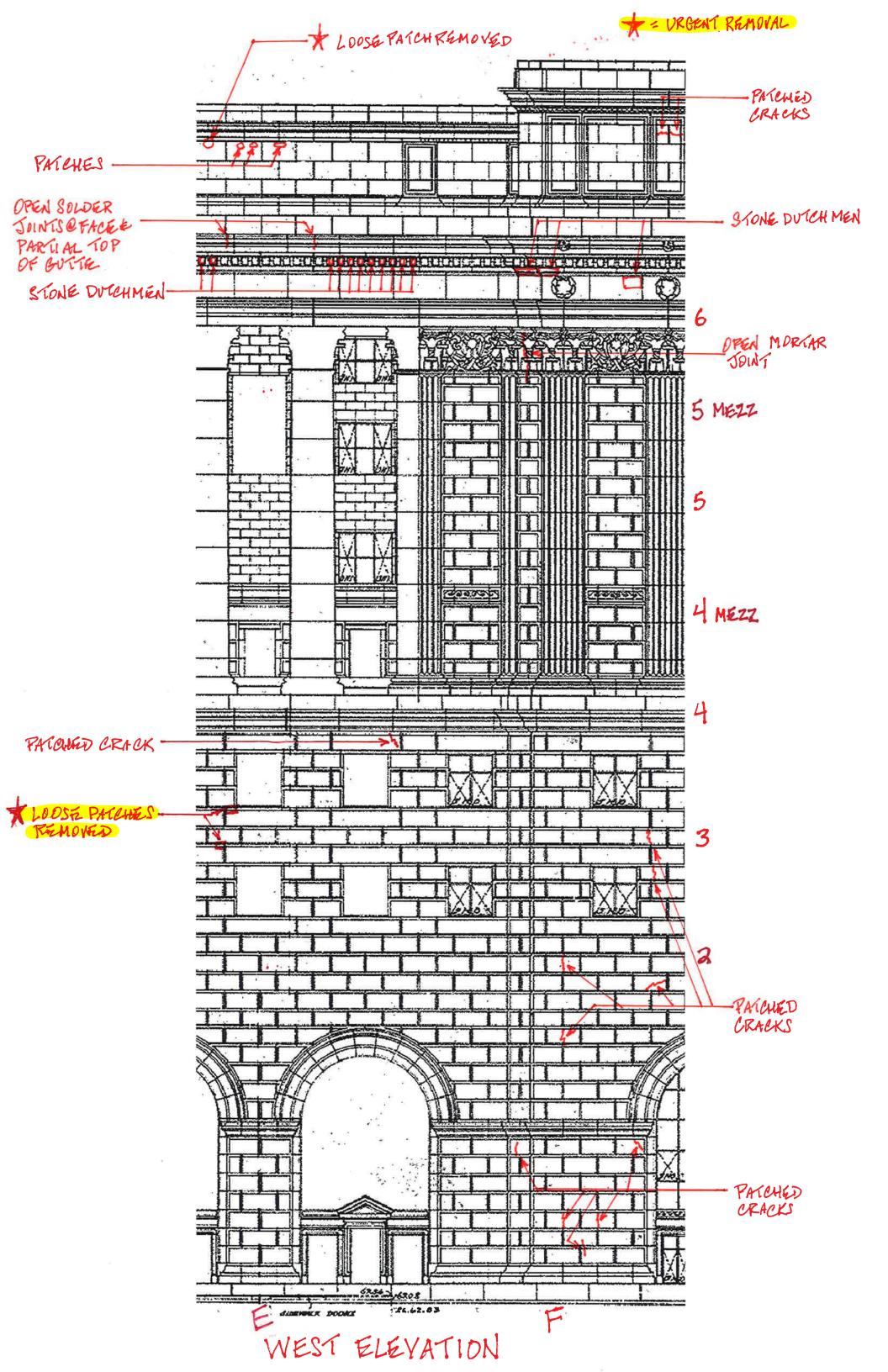


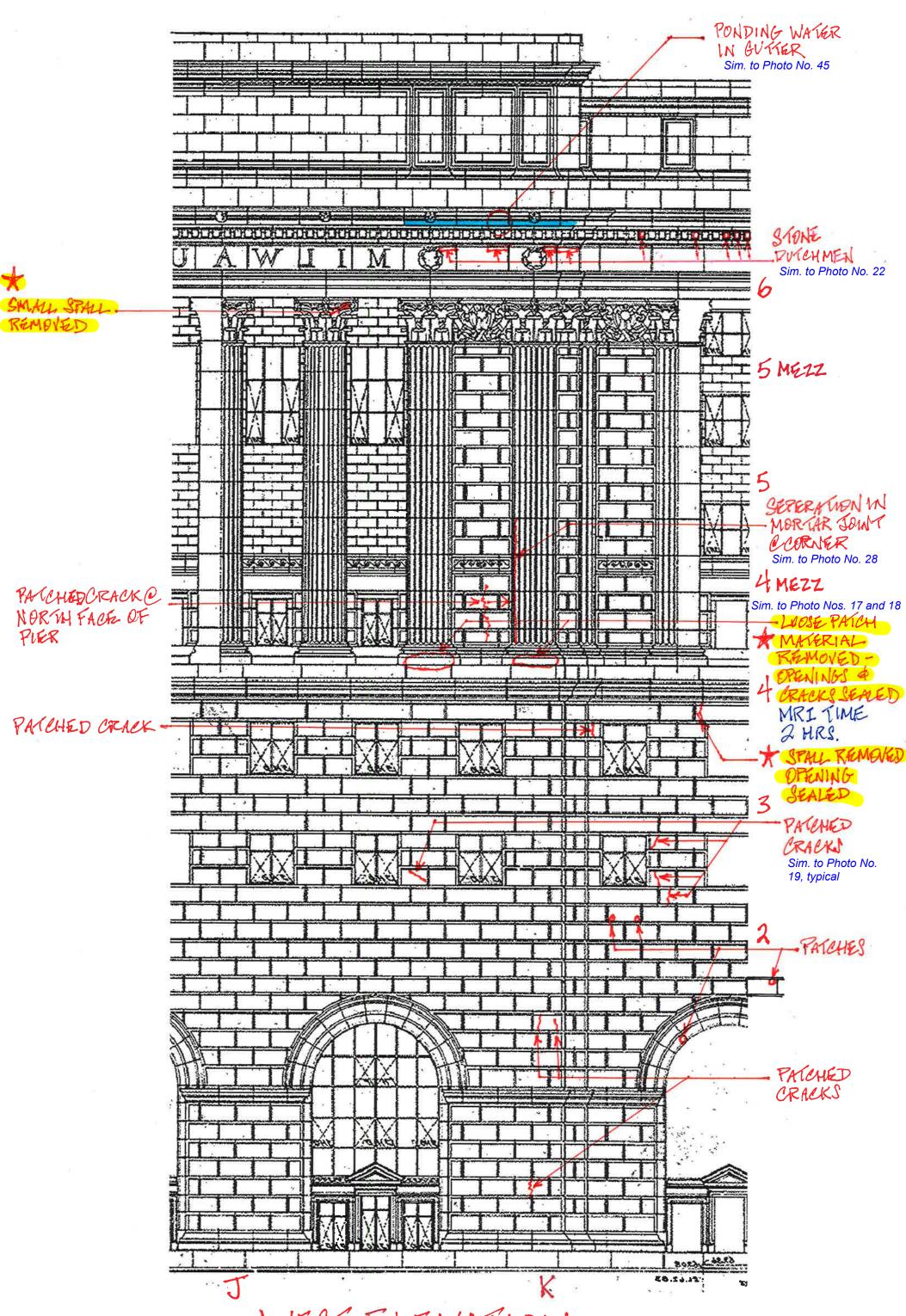
4 N. 1



WEST ELEVATION

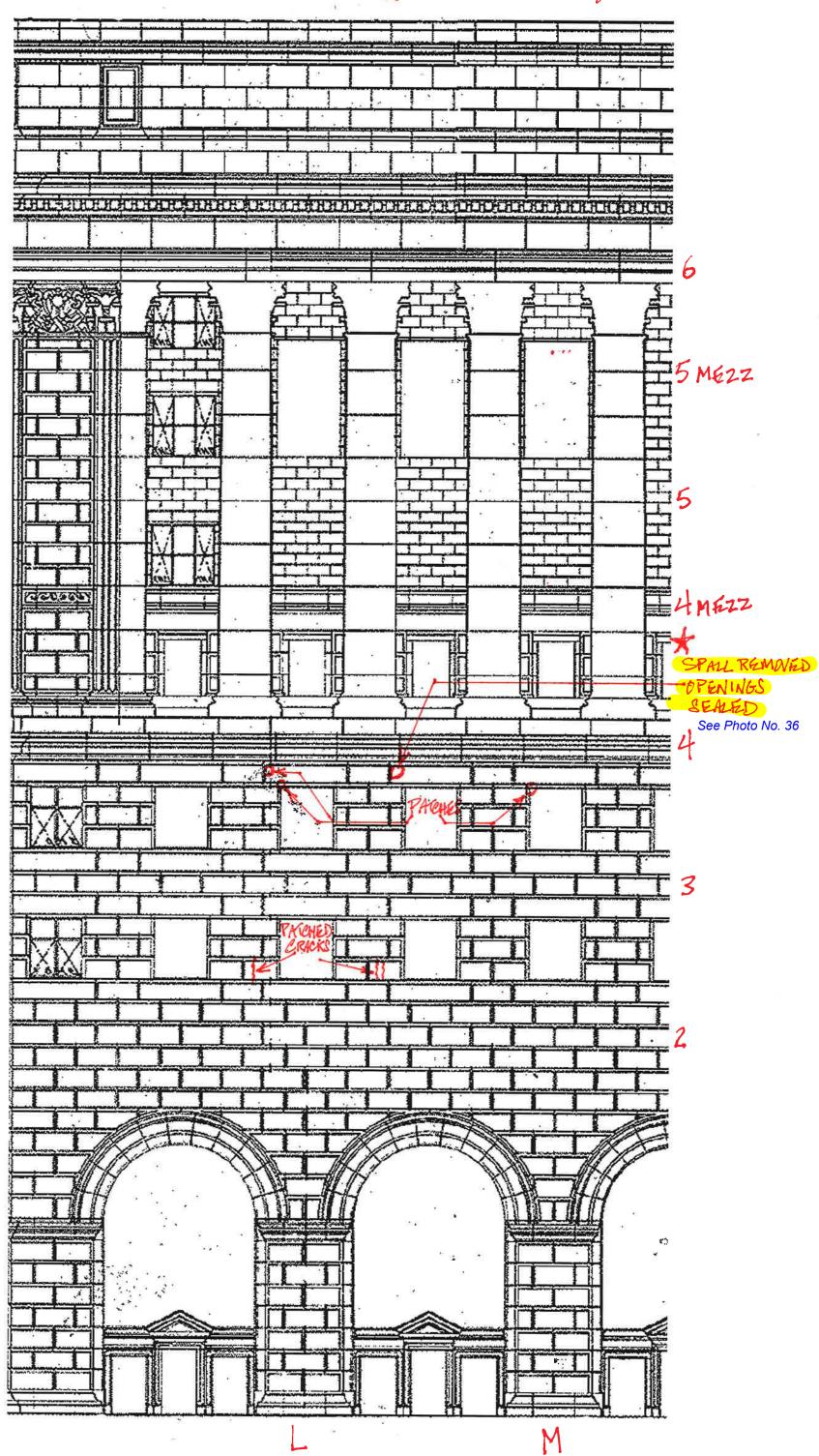






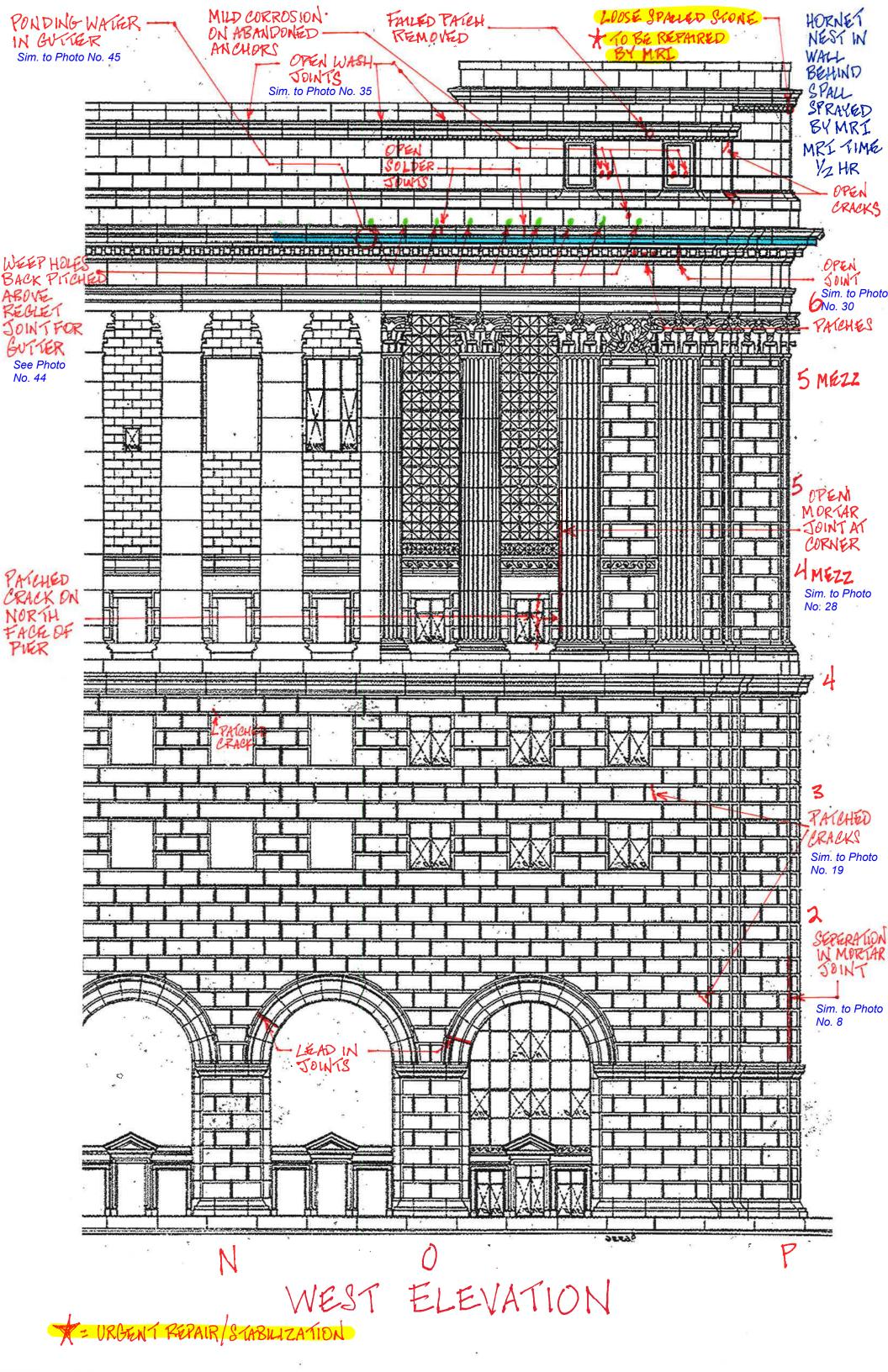
WEST ELEVATION

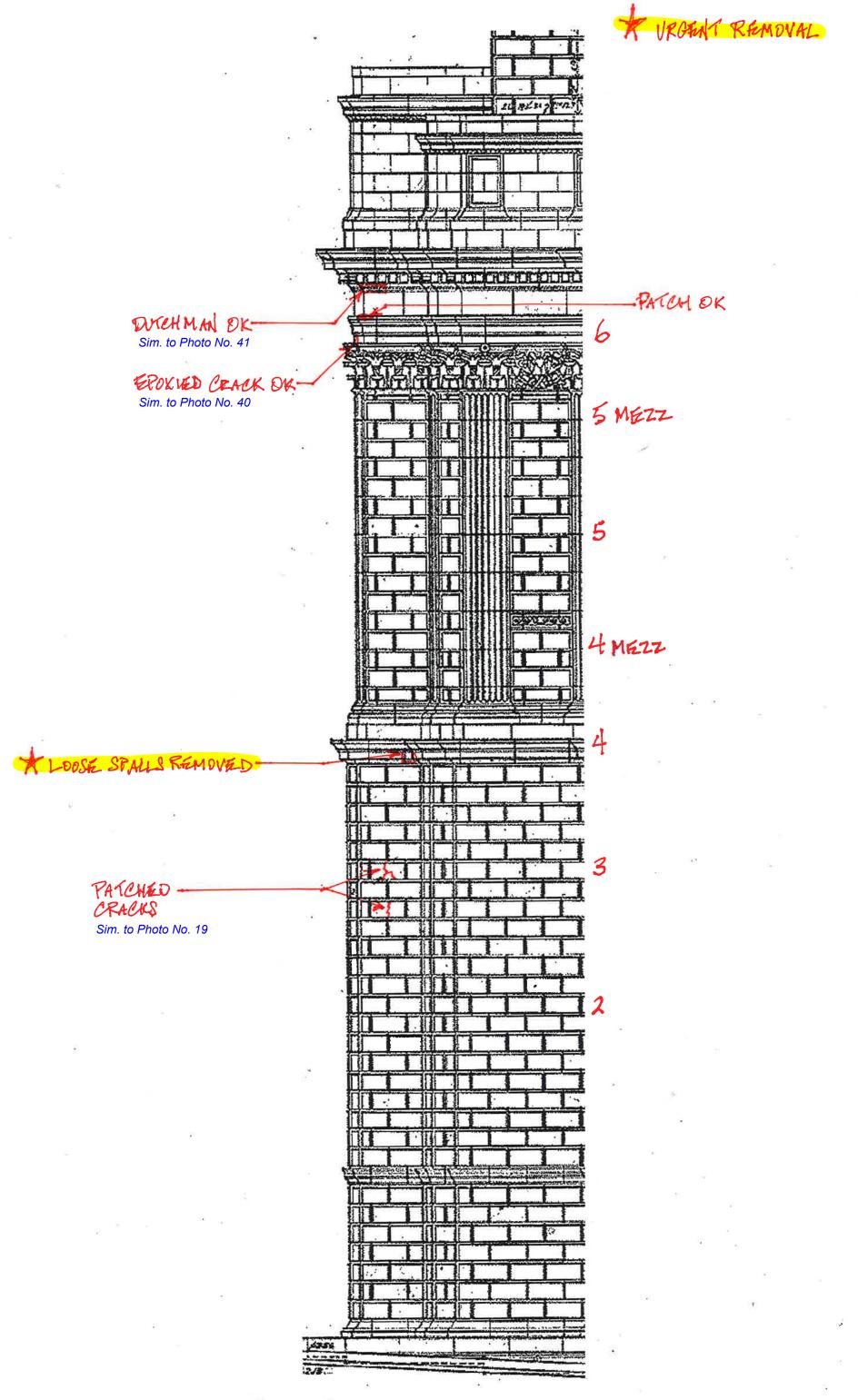
TOURGENT REMOVAL/REPAIR



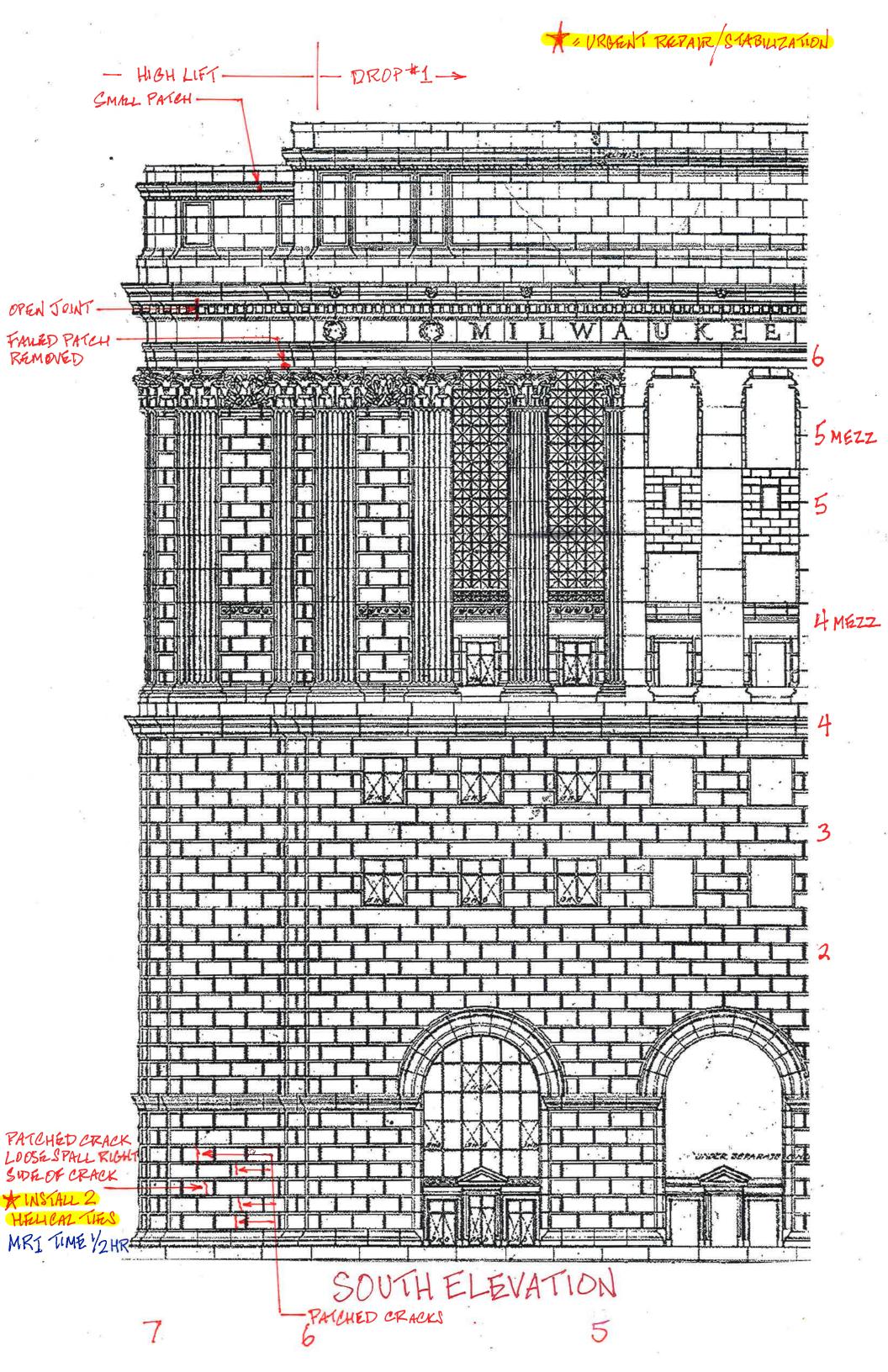
K

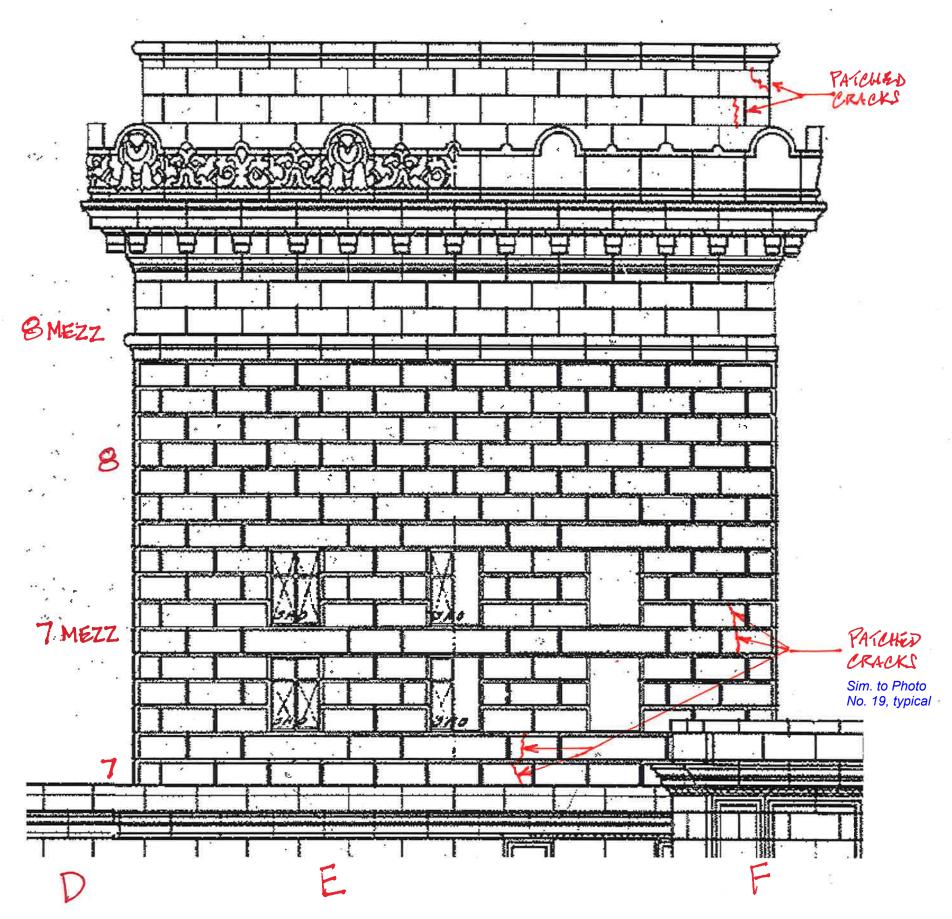
WEST ELEVATION



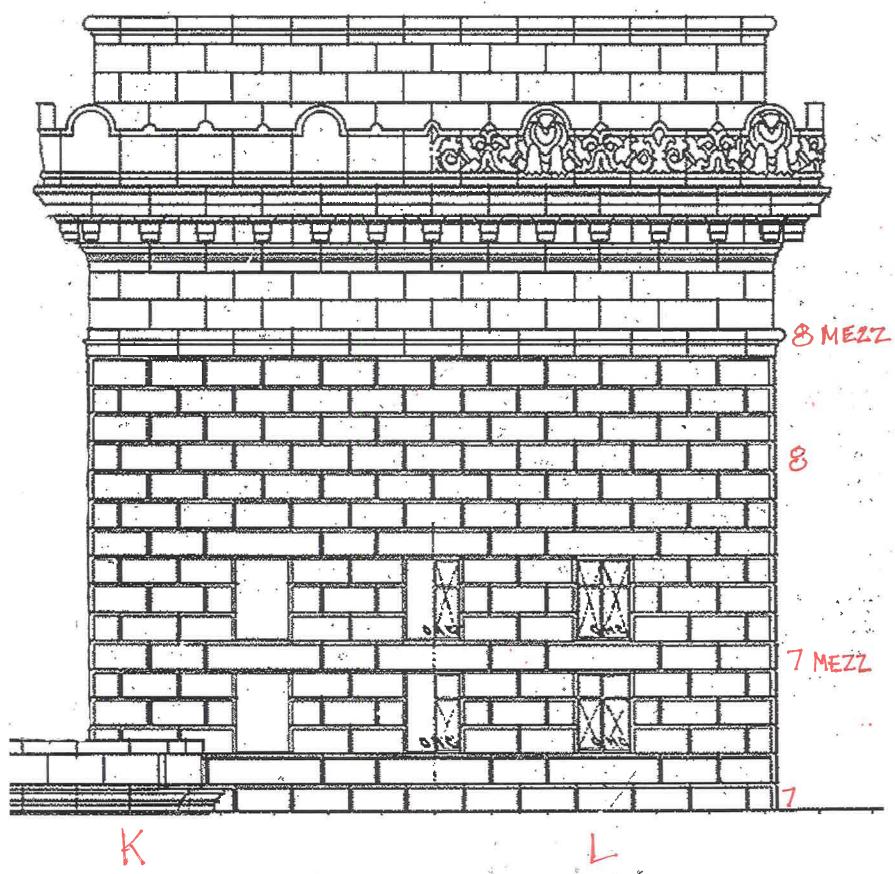


SOUTH SET BACK-WEST ELEVATION





WEST ELEVATION



WEST ELEVATION.

APPENDIX D

Representative Photographs

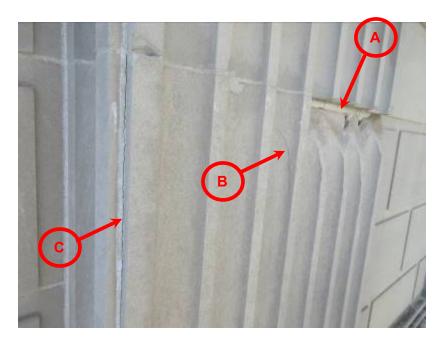


Photo No. 1 (A) Previously removed spall and (B) new cracks in stone unit on Drop #18b. Note (C) separation in mortar joint.

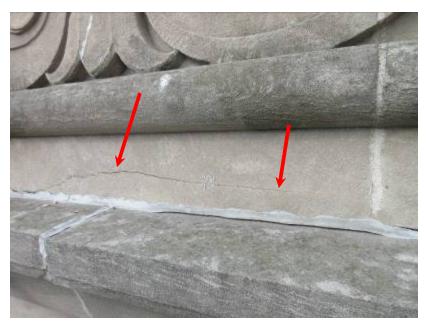


Photo No. 2 Spalls at the eighth floor cornice. Spall was sealed.



Photo No. 3 Spall removed at sixth floor cornice at drop no. 17.



Photo No. 4 Spall removed and open joint sealed at the ashlar base of the building at drop no. 15.



Photo No. 5 View of corroded cramp tie after removal of spall at southeast corner below eighth floor cornice. Cramp and open joints were sealed.

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Photo No. 6 Cracking/spalling visible at pier flanking the revolving door entrance at the east elevation.

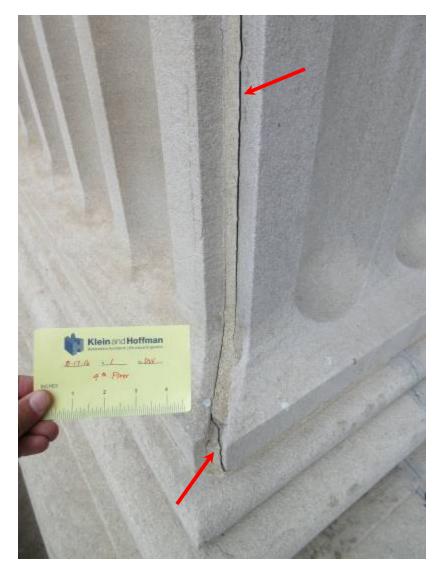
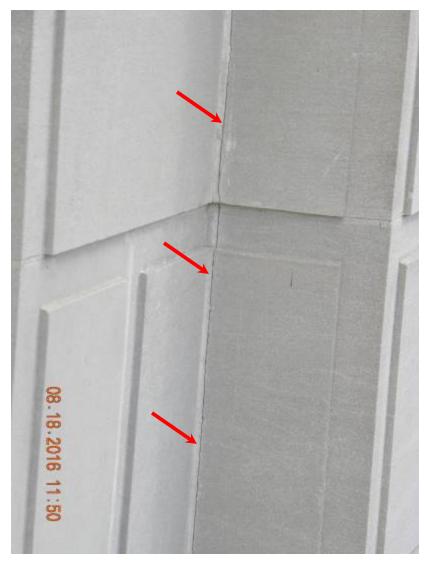


Photo No. 7 Separation in mortar joint caused by ongoing movement of the stone units.



Separation in mortar joint caused by ongoing Photo No. 8 movement of the stone units.

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Photo No. 9 View of open joints and shifted masonry at northeast corner parapet.



Photo No. 10 Slight displacement of cornice stone at the southeast corner of the eighth floor cornice is visible. Note failed horizontal sealant joint.

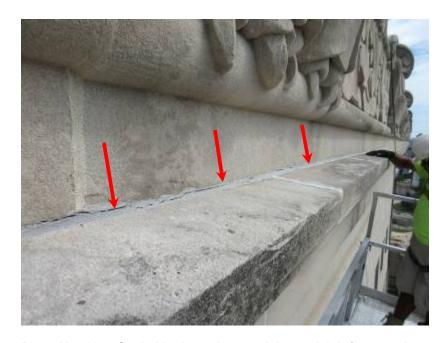


Photo No. 11 Sealed horizontal mortar joint at eighth floor cornice.

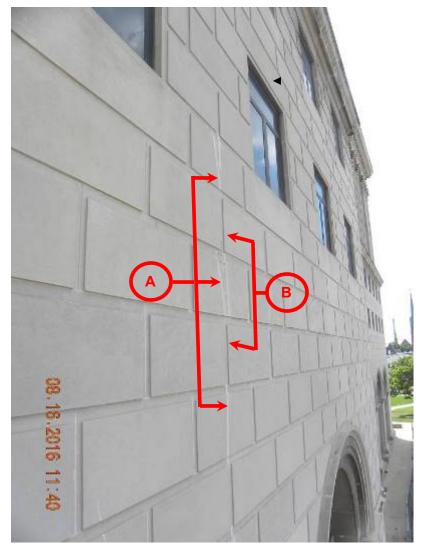


Photo No. 12 Continuous cracks (A) and separations in vertical mortar joints between cracked units (B) indicate ongoing distress within the walls.

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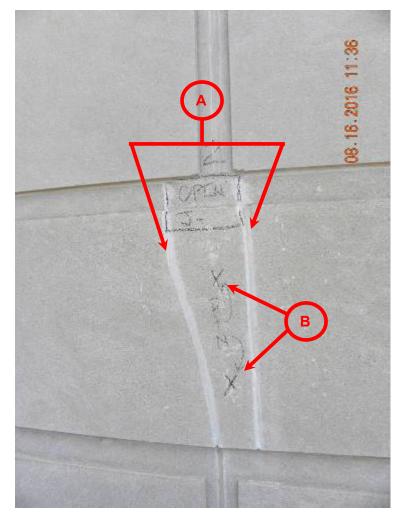


Photo No. 13 Previously patched cracks, with patch material that does not match the stonework, have started to separate (A). The loose portion of that patched stone was designated to be pinned with stainless steel pins as temporary stabilization (B).

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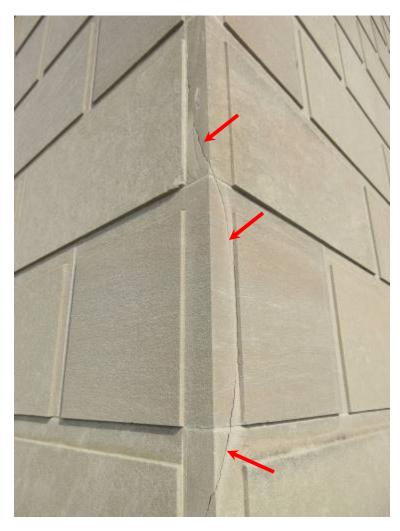


Photo No. 14 Cracked stones at the seventh floor at the southeast corner.

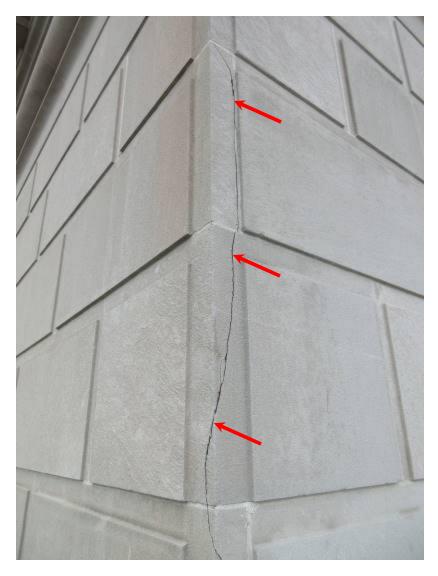


Photo No. 15 Cracked stones at the eighth floor at the northeast corner.



Photo No. 16 Cracked stone at return of ground level arcade.



Photo No. 17 Cracked and delaminated previously applied patch on west elevation, J-K, was designated for removal during our inspections.



Photo No. 18 Removal of defective patch material from previous photo and sealing of openings. No supplemental reinforcing was found in the previous patches removed during our inspections.

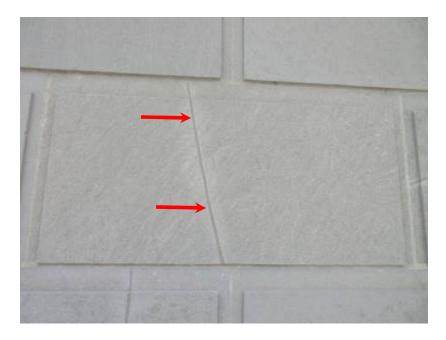


Photo No. 19 Typical previous crack repair in good condition.

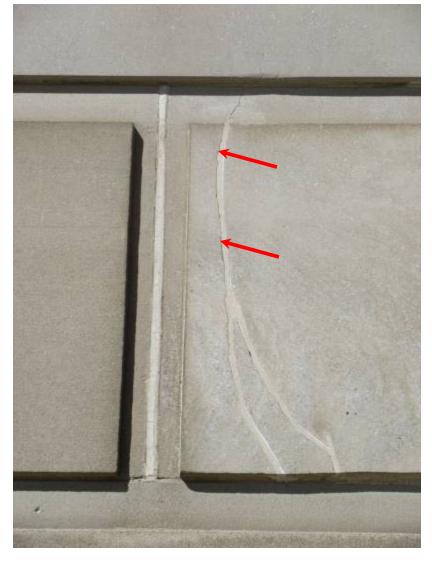


Photo No. 20 New cracks developing at previously repaired crack.

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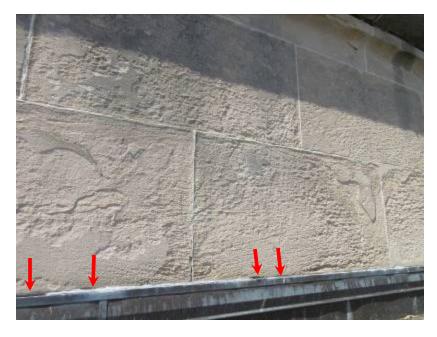
Photo No. 21 Previous pin and epoxy repairs at drop no. 10. Pins (A) located on the east face of the wall, epoxied crack (B) located on the south face of the wall.



Photo No. 22 Previous pin and epoxy repair at a dentil at the sixth floor cornice at drop no. 3.



Photo No. 23 Cracked stone at the base of a column at drop no. 14.



Pitted stone surfaces possibly caused by harsh acids allowed to dwell for extended periods on the Photo No. 24 façades during previous cleaning efforts. Note large voids in the sealant joints at the top of the roof counterflashing.



Photo No. 25 Widespread rust staining on the east elevation likely caused by cleaning chemicals reacting with the iron in the limestone.



Weathered stone surface with minor delamination at Photo No. 26 the eighth floor cornice of the east elevation. Surface delaminations were removed where observed.



Photo No. 27 Showing typical condition of ashlar limestone and mortar joints.

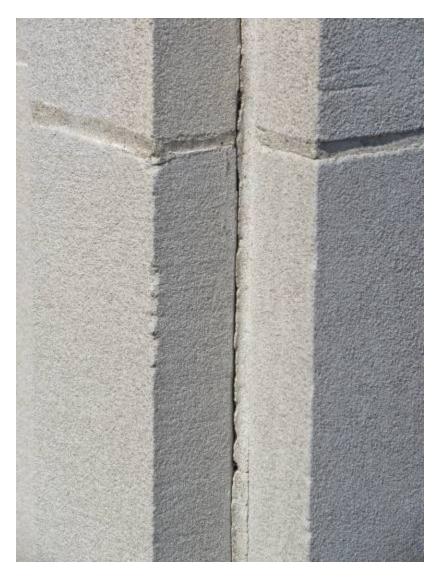


Photo No. 28 Typical deteriorated mortar joint at corner of pilaster.

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Photo No. 29 Failed mortar joint at corner of ground level arched opening at drop no. 16.



Photo No. 30 Mortar joint at upper cornice at south end of west elevation in poor condition with open voids and separations.



Fairly recently tuck pointed mortar joints do not match the existing mortar joints. Photo No. 31



Photo No. 32 Deterioration of the stone surface at the underside of sixth floor cornice.



Photo No. 33 Showing significant deterioration of stonework at south entrance.



Photo No. 34 Open sealant joint at base of parapet wall on Drop #3.

Photo Nos. 13 & 14: Aged, failing, and altoge wash (upward facing) joi throughout the façades.



Photo No. 35 Missing sealant joint on 4th floor wash coursing on Drop #17.



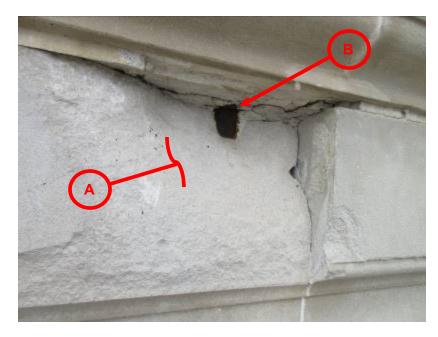
Photo No. 36 Fairly large, loose spall removed directly below 4th floor wash coursing, L – M Façade, west elevation.



Photo No. 37 Close-up photo of embedded shelf angle above removed spall in previous photo. Lack of corrosion on the angle indicates that the spall was caused by moisture trapped in the wall during freeze/thaw cycles.



Photo No. 38 Spalled stone caused by corrosion on outer edge of embedded support angle at first floor, Drop 20.



(A) Large spalled section of stone caused by corrosion of embedded cramp anchor (B) (lateral tie for the stone) at 6th floor on Drop no. 16. Photo No. 39



Photo No. 40 View of epoxy repair at east main entrance at drop no. 12

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Photo No. 41 View of typical stone Dutchman repair.



Photo No. 42 Open seam in side of gutter, E – F, west elevation.



Photo No. 43 Open seam in top of gutter at Drop #1.

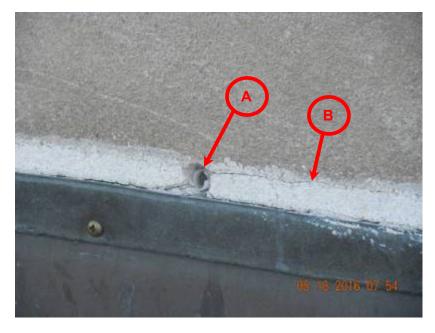


Photo No. 44 (A) Back-pitched weep hole above gutter at 6^{th} floor, N - P Façades, west elevation. Note deteriorated sealant joint (B).

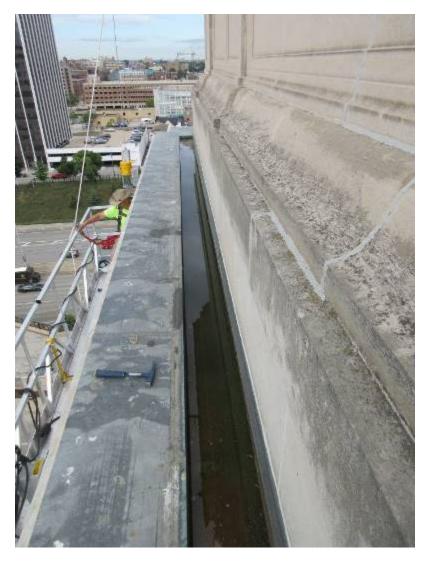


Photo No. 45 Ponding water in gutter at west end of south elevation. Extends around the corner on the west elevation.



Photo No. 46 Open and deteriorated sealant joints surrounding the metal collector pans at Drop #2 at the 4th floor.



Photo No. 47 Showing typical failed window perimeter sealant joint.



Photo No. 48 Cracked brickwork at the 6th floor level, Drop no. 21, was sealed during inspections.

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Photo No. 49 Small crack at sixth floor northeast parapet.



Photo No. 50 Small spall at sixth floor southeast parapet.



Multiple spalls and cracks were observed at the sixth Photo No. 51 floor northeast corner parapet.



Showing weathered stone at high roof parapet. Photo no. 52



Photo No. 53 View of small spall cause by sheet metal flashing fastener.



Photo No. 54 Showing failed sealant joint at north seventh floor parapet.

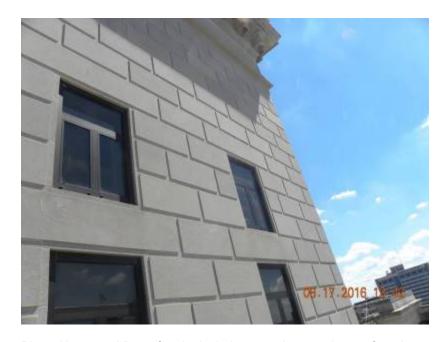
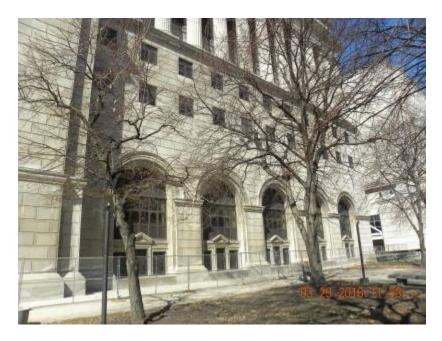


Photo No. 55 View of typical windows on the courthouse façades.



Photo No. 56 View of typical grillage over masonry and windows at the upper floor of the colonnades.



East Elevation – showing large set back ornamental windows with faux entry portals at the first floor of Photo No. 57. the courthouse.



Showing original bronze windows at east elevation Photo No. 58 main entrance.



Photo No. 59 Stainless steel straps installed at southeast corner of seventh floor for stabilization of cracked stones.



Photo No. 60 Stainless steel pins installed at cracked stones at the northeast sixth floor parapet.



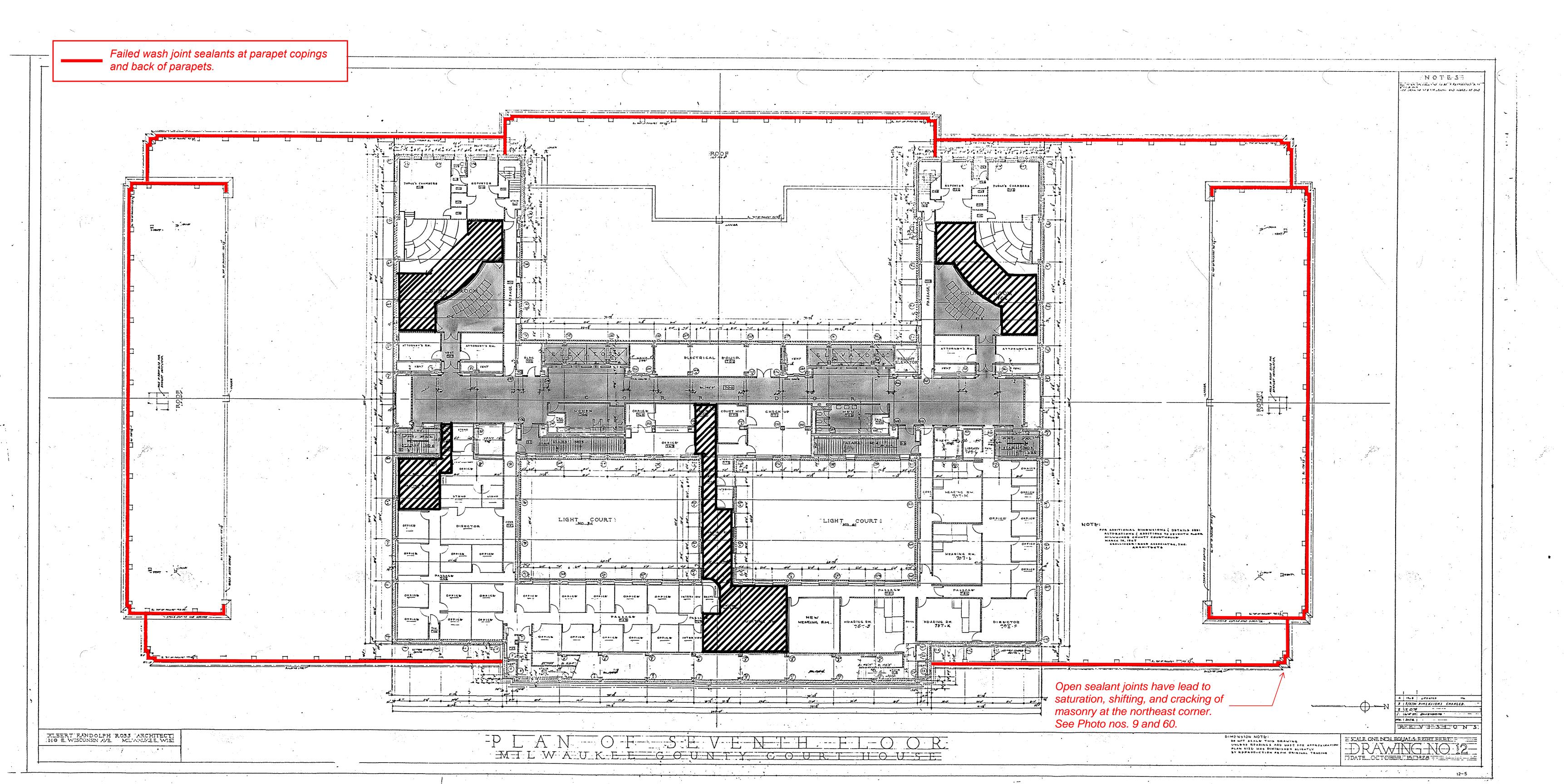
Photo No. 61 Spall to be removed. Black "X's" indicate specified locations for installation of pins.

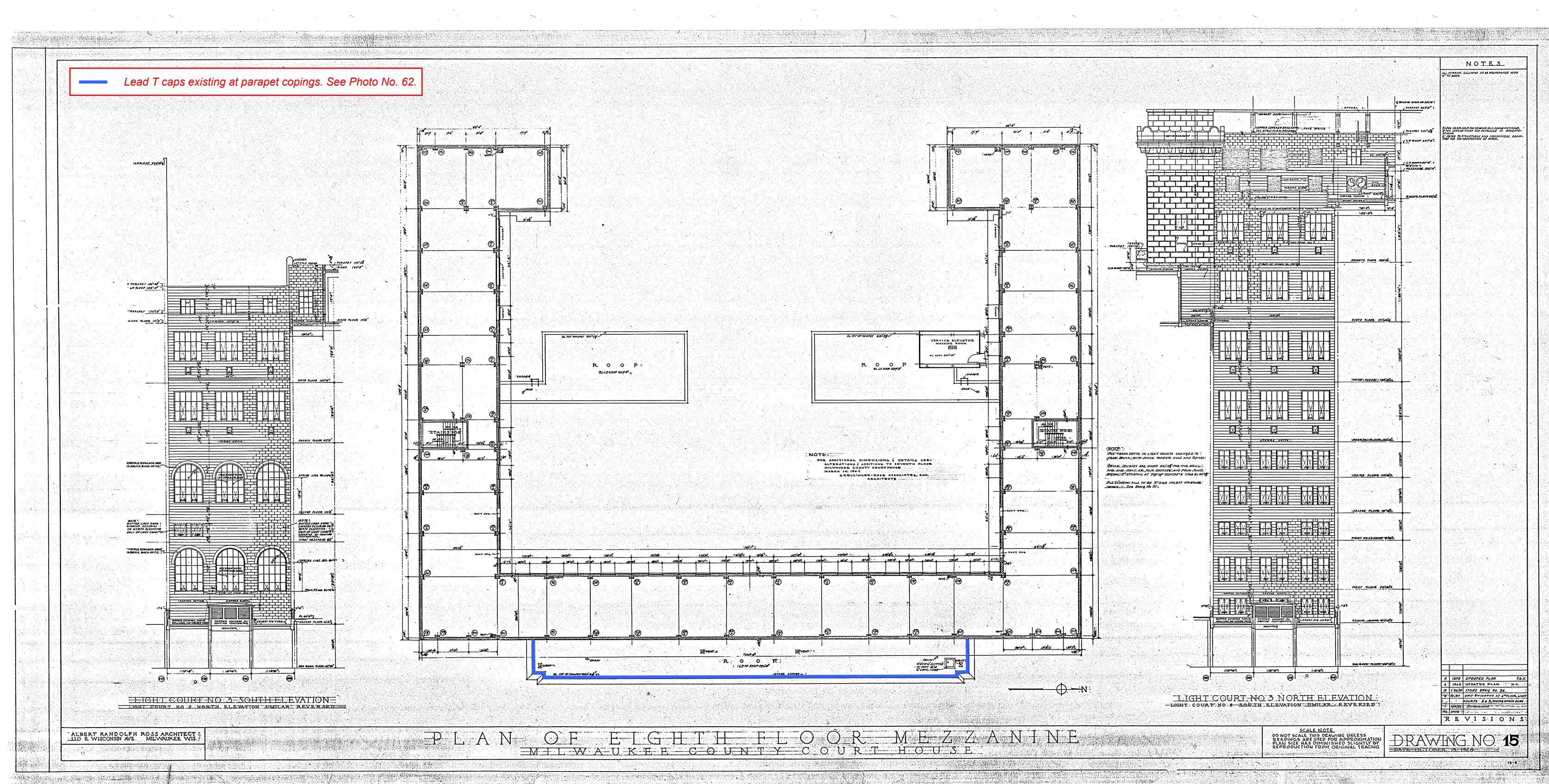


Weather cap joint protection installed on the wash joints of the coping stone on Drop #22. Photo No. 62

APPENDIX E

Sealant Condition Plans





Failed wash joint sealants at stone-to-stone joints and where copper gutter is regletted into stone at 8th floor cornice. See Photo No. 11. Failed wash joint sealants at parapet copings. Sim. to Photo No. 46. NOTES Slight outward displacement and joint separation indicate movement of cornice stones at corner. Sim. to Photo No. 10 R 0 0 F FOR ADDITIONAL DIMENSIONS [DETAILS SERVING ALTERATIONS] ADDITIONS TO SEVENTH PLOOR MILWAUKER COUNTY COURTHOUSE MARCH 10, 1967...
GRELLINGER - ROSE ASSOCIATES, INC.
ADGRITHECTS TOOP NE COURSE THE STATE OF THE STAT S. PA OF PRINTE BLANGS AL TO OF PLLATE DIFFE R OK OF R. OF ASOF ESTASE 5 1979 UPDATED PLAN T.G.R
4 1968 UPDATED PLAN M.C.
3 6/24/30 STONE DRWG, NO. 24.
E 16/60 TINDOW HEIGHT GROFFE LIGHT COUR
1 1/2/27 DHENSIONS
NO. DATE

R. B. V. I. S. I. O. N. S. Slight outward displacement and joint LIGHT-COURT NO 3 EAST ELEVATION : LIGHT COURT NO 2 BAST ELEVATION SIMILAR REVERSED Joint separation indicates movement of cornice separation indicate movement of cornice stones at corner. Sim. to Photo No. 10. stones at corner. See Photo No. 10. ALBERT RANDOLPH ROSS ARCHITECT DRAWING NO 16
DATE OCTOBER 15, 1928

APPENDIX F

Sixth Floor Gutter Condition Plans

